

APPENDIX I

Level of Service Calculations

HCS+™ DETAILED REPORT

General Information				Site Information			
Analyst	JAG			Intersection	Route 94/Rt 17 SB ramp		
Agency or Co.	TMA			Area Type	All other areas		
Date Performed	2/20/09			Jurisdiction	Village of Chester		
Time Period	AM Peak Hour			Analysis Year	Existing Condition		
				Project ID			

Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N _l		1	1	1	1					1	1	0	
Lane Group		T	R	L	T					L	TR		
Volume, V (vph)		80	260	238	96					253	1	291	
% Heavy Vehicles, %HV		8	8	5	5					5	5	5	
Peak-Hour Factor, PHF		0.94	0.94	0.78	0.78					0.71	0.71	0.71	
Pretimed (P) or Actuated (A)		A	A	A	A					A	A	A	
Start-up Lost Time, l ₁		2.0	2.0	2.0	2.0					2.0	2.0		
Extension of Effective Green, e		2.0	2.0	2.0	2.0					2.0	2.0		
Arrival Type, AT		3	3	5	5					3	3		
Unit Extension, UE		3.0	3.0	3.0	3.0					3.0	3.0		
Filtering/Metering, I		1.000	1.000	1.000	1.000					1.000	1.000		
Initial Unmet Demand, Q _b		0.0	0.0	0.0	0.0					0.0	0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0	
Lane Width		12.0	13.0	12.0	12.0					11.0	12.0		
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	5	N	
Parking Maneuvers, N _m													
Buses Stopping, N _b		0	0	0	0					0	0		
Min. Time for Pedestrians, G _p		3.2			3.2			3.2			3.2		
Phasing	EW Perm	WB Only	03		04		SB Only	06		07		08	
Timing	G = 38.0	G = 5.0	G =		G =		G = 42.0	G =		G =		G =	
	Y = 5	Y = 5	Y =		Y =		Y = 5	Y =		Y =		Y =	
Duration of Analysis, T = 0.25							Cycle Length, C = 100.0						

Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		85	277	305	123					356	411	
Lane Group Capacity, c		668	587	655	869					680	630	
v/c Ratio, X		0.13	0.47	0.47	0.14					0.52	0.65	
Total Green Ratio, g/C		0.38	0.38	0.48	0.48					0.42	0.42	
Uniform Delay, d ₁		20.2	23.4	17.8	14.5					21.6	23.2	
Progression Factor, PF		1.000	1.000	0.926	0.385					1.000	1.000	
Delay Calibration, k		0.11	0.11	0.11	0.11					0.13	0.23	
Incremental Delay, d ₂		0.1	0.6	0.5	0.1					0.7	2.4	
Initial Queue Delay, d ₃		0.0	0.0	0.0	0.0					0.0	0.0	
Control Delay		20.3	24.0	17.0	5.7					22.3	25.6	
Lane Group LOS		C	C	B	A					C	C	
Approach Delay	23.1			13.7						24.1		
Approach LOS	C			B						C		
Intersection Delay	21.0			X _C = 0.57			Intersection LOS			C		

HCS+™ DETAILED REPORT

General Information				Site Information			
Analyst	JAG	Intersection	Rt 94/Rt 17 NB ramps	Area Type	All other areas		
Agency or Co.	TMA	Jurisdiction	Village of Chester	Analysis Year	Existing Condition		
Date Performed	2/20/09	Project ID					
Time Period	AM Peak Hour						

Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _i	1	1			1	1	0	1	1			
Lane Group	L	T			T	R		LT	R			
Volume, V (vph)	60	314			307	317	97	1	83			
% Heavy Vehicles, %HV	5	5			2	2	8	8	8			
Peak-Hour Factor, PHF	0.91	0.91			0.94	0.94	0.77	0.77	0.77			
Pretimed (P) or Actuated (A)	A	A			A	A	A	A	A			
Start-up Lost Time, l _i	2.0	2.0			2.0	2.0		2.0	2.0			
Extension of Effective Green, e	2.0	2.0			2.0	2.0		2.0	2.0			
Arrival Type, AT	5	5			3	3		3	3			
Unit Extension, UE	3.0	3.0			3.0	3.0		3.0	3.0			
Filtering/Metering, I	1.000	1.000			1.000	1.000		1.000	1.000			
Initial Unmet Demand, Q _b	0.0	0.0			0.0	0.0		0.0	0.0			
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0	0	0	0	
Lane Width	12.0	12.0			11.0	13.0		12.0	13.0			
Parking / Grade / Parking	N	0	N	N	6	N	N	5	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0			0	0		0	0			
Min. Time for Pedestrians, G _p	3.2			3.2			3.2			3.2		
Phasing	EW Perm	EB Only	03	04	NB Only	06	07	08				
Timing	G = 50.0	G = 5.0	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 100.0					

Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	66	345			327	337		127	108			
Lane Group Capacity, c	619	1086			874	794		490	452			
v/c Ratio, X	0.11	0.32			0.37	0.42		0.26	0.24			
Total Green Ratio, g/C	0.60	0.60			0.50	0.50		0.30	0.30			
Uniform Delay, d ₁	11.4	9.9			15.4	15.9		26.6	26.4			
Progression Factor, PF	0.935	0.125			1.000	1.000		1.000	1.000			
Delay Calibration, k	0.11	0.11			0.11	0.11		0.11	0.11			
Incremental Delay, d ₂	0.1	0.2			0.3	0.4		0.3	0.3			
Initial Queue Delay, d ₃	0.0	0.0			0.0	0.0		0.0	0.0			
Control Delay	10.7	1.4			15.6	16.2		26.8	26.7			
Lane Group LOS	B	A			B	B		C	C			
Approach Delay	2.9			15.9			26.8					
Approach LOS	A			B			C					
Intersection Delay	13.8			X _C = 0.32			Intersection LOS			B		

HCS+™ DETAILED REPORT

General Information	Site Information
Analyst <i>JAG</i>	Intersection <i>NYS Rt. 94/ NYS Rt. 17M</i>
Agency or Co. <i>TMA</i>	Area Type <i>All other areas</i>
Date Performed <i>2/20/09</i>	Jurisdiction <i>Village of Chester</i>
Time Period <i>AM Peak Hour</i>	Analysis Year <i>Existing Condition</i>
	Project ID

Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N ₁	1	1	1	1	1	0	2	1	0	1	2	0	
Lane Group	L	LT	R	L	TR		L	TR		L	TR		
Volume, V (vph)	78	83	170	38	153	57	273	232	8	36	172	175	
% Heavy Vehicles, %HV	13	13	13	6	6	6	6	6	6	7	7	7	
Peak-Hour Factor, PHF	0.83	0.83	0.83	0.86	0.86	0.86	0.92	0.92	0.92	0.86	0.86	0.86	
Pretimed (P) or Actuated (A)	A	A	A	A	A	A	A	A	A	A	A	A	
Start-up Lost Time, l ₁	2.0	2.0	2.0	2.0	2.0		2.0	2.0		2.0	2.0		
Extension of Effective Green, e	2.0	2.0	2.0	2.0	2.0		2.0	2.0		2.0	2.0		
Arrival Type, AT	3	3	3	3	3		3	3		3	3		
Unit Extension, UE	3.0	3.0	3.0	3.0	3.0		3.0	3.0		3.0	3.0		
Filtering/Metering, I	1.000	1.000	1.000	1.000	1.000		1.000	1.000		1.000	1.000		
Initial Unmet Demand, Q _b	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0	
Lane Width	12.0	12.0	12.0	12.0	12.0		10.0	10.0		10.0	12.0		
Parking / Grade / Parking	N	-5	N	N	-3	N	N	-5	N	N	5	N	
Parking Maneuvers, N _m													
Buses Stopping, N _b	0	0	0	0	0		0	0		0	0		
Min. Time for Pedestrians, G _p	3.2			3.2			3.2			3.2			
Phasing	WB Only	EB Only	03			04			Thru & RT	NB Only	SB Only	08	
Timing	G = 20.0	G = 19.0	G =			G =			G = 14.0	G = 15.0	G = 12.0	G =	
	Y = 4	Y = 4	Y =			Y =			Y = 4	Y = 4	Y = 4	Y =	
Duration of Analysis, T = 0.25							Cycle Length, C = 100.0						

Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	94	100	205	44	244		297	261		42	403	
Lane Group Capacity, c	311	327	278	346	349		475	563		184	792	
v/c Ratio, X	0.30	0.31	0.74	0.13	0.70		0.63	0.46		0.23	0.51	
Total Green Ratio, g/C	0.19	0.19	0.19	0.20	0.20		0.15	0.33		0.12	0.26	
Uniform Delay, d ₁	34.8	34.8	38.2	32.8	37.2		39.9	26.5		39.8	31.6	
Progression Factor, PF	1.000	1.000	1.000	1.000	1.000		1.000	1.000		1.000	1.000	
Delay Calibration, k	0.11	0.11	0.30	0.11	0.27		0.21	0.11		0.11	0.12	
Incremental Delay, d ₂	0.6	0.5	9.9	0.2	6.1		2.6	0.6		0.6	0.5	
Initial Queue Delay, d ₃	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Control Delay	35.4	35.4	48.1	33.0	43.3		42.5	27.1		40.4	32.1	
Lane Group LOS	D	D	D	C	D		D	C		D	C	
Approach Delay	41.9			41.7			35.3			32.9		
Approach LOS	D			D			D			C		
Intersection Delay	37.3			X _C = 0.60			Intersection LOS			D		

TWO-WAY STOP CONTROL SUMMARY

General Information			Site Information					
Analyst	JAG		Intersection	Hambletonian and High				
Agency/Co.	TMA		Jurisdiction	Chester				
Date Performed	2/17/2009		Analysis Year	Existing Condition				
Analysis Time Period	AM Peak Hour							
Project Description								
East/West Street: <i>Hambletonian Avenue</i>			North/South Street: <i>High Street</i>					
Intersection Orientation: <i>North-South</i>			Study Period (hrs): <i>0.25</i>					
Vehicle Volumes and Adjustments								
Major Street	Northbound			Southbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	103	174			309	116		
Peak-Hour Factor, PHF	0.95	0.95	1.00	1.00	0.89	0.89		
Hourly Flow Rate, HFR (veh/h)	108	183	0	0	347	130		
Percent Heavy Vehicles	10	--	--	0	--	--		
Median Type	<i>Undivided</i>							
RT Channelized			0			0		
Lanes	0	1	0	0	1	0		
Configuration	LT					TR		
Upstream Signal		0			0			
Minor Street	Eastbound			Westbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)	54		99					
Peak-Hour Factor, PHF	0.60	1.00	0.60	1.00	1.00	1.00		
Hourly Flow Rate, HFR (veh/h)	89	0	164	0	0	0		
Percent Heavy Vehicles	12	0	12	0	0	0		
Percent Grade (%)	0			0				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	0		
Configuration		LR						
Delay, Queue Length, and Level of Service								
Approach	Northbound	Southbound	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	LT						LR	
v (veh/h)	108						253	
C (m) (veh/h)	1045						451	
v/c	0.10						0.56	
95% queue length	0.34						3.37	
Control Delay (s/veh)	8.8						22.7	
LOS	A						C	
Approach Delay (s/veh)	--	--					22.7	
Approach LOS	--	--					C	

TWO-WAY STOP CONTROL SUMMARY								
General Information				Site Information				
Analyst	JAG			Intersection	Ward and Rt 17M			
Agency/Co.	TMA			Jurisdiction	Goshen			
Date Performed	2/17/2009			Analysis Year	Existing Condition			
Analysis Time Period	AM Peak Hour							
Project Description								
East/West Street: Ward Road				North/South Street: NYS Route 17M				
Intersection Orientation: North-South				Study Period (hrs): 0.25				
Vehicle Volumes and Adjustments								
Major Street	Northbound			Southbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)		192	3	24	192			
Peak-Hour Factor, PHF	1.00	0.87	0.87	0.92	0.92	1.00		
Hourly Flow Rate, HFR (veh/h)	0	220	3	26	208	0		
Percent Heavy Vehicles	0	--	--	1	--	--		
Median Type	Undivided							
RT Channelized			0			0		
Lanes	0	1	0	0	1	0		
Configuration			TR	LT				
Upstream Signal		0			0			
Minor Street	Eastbound			Westbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)				3		65		
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.89	1.00	0.89		
Hourly Flow Rate, HFR (veh/h)	0	0	0	3	0	73		
Percent Heavy Vehicles	0	0	0	1	0	1		
Percent Grade (%)		0			0			
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	0		
Configuration					LR			
Delay, Queue Length, and Level of Service								
Approach	Northbound	Southbound	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration		LT		LR				
v (veh/h)		26		76				
C (m) (veh/h)		1352		803				
v/c		0.02		0.09				
95% queue length		0.06		0.31				
Control Delay (s/veh)		7.7		10.0				
LOS		A		A				
Approach Delay (s/veh)	--	--	10.0					
Approach LOS	--	--	A					

TWO-WAY STOP CONTROL SUMMARY								
General Information				Site Information				
Analyst	JAG			Intersection	Rt 17M and Main			
Agency/Co.	TMA			Jurisdiction	Chester			
Date Performed	2/17/2009			Analysis Year	Existing Condition			
Analysis Time Period	AM Peak Hour							
Project Description								
East/West Street: NYS Route 17M				North/South Street: Main Street				
Intersection Orientation: East-West				Study Period (hrs): 0.25				
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	5	272			439	75		
Peak-Hour Factor, PHF	0.94	0.94	1.00	1.00	0.85	0.85		
Hourly Flow Rate, HFR (veh/h)	5	289	0	0	516	88		
Percent Heavy Vehicles	7	--	--	0	--	--		
Median Type	Two Way Left Turn Lane							
RT Channelized			0			0		
Lanes	1	1	0	0	1	0		
Configuration	L	T				TR		
Upstream Signal		0			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)				97		12		
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.91	1.00	0.91		
Hourly Flow Rate, HFR (veh/h)	0	0	0	106	0	13		
Percent Heavy Vehicles	0	0	0	1	0	1		
Percent Grade (%)	0			0				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	0		
Configuration					LR			
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	L						LR	
v (veh/h)	5						119	
C (m) (veh/h)	950						506	
v/c	0.01						0.24	
95% queue length	0.02						0.91	
Control Delay (s/veh)	8.8						14.3	
LOS	A						B	
Approach Delay (s/veh)	--	--					14.3	
Approach LOS	--	--					B	

HCS+™ DETAILED REPORT

General Information				Site Information			
Analyst	JAG			Intersection	Arcadia and Rt 17M		
Agency or Co.	TMA			Area Type	All other areas		
Date Performed	2/17/2009			Jurisdiction	Goshen		
Time Period	AM Peak Hour			Analysis Year	Existing Condition		
				Project ID			

Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0		0				0	1			1	0
Lane Group		LR						LT			TR	
Volume, V (vph)	48		47				23	229			188	25
% Heavy Vehicles, %HV	1		1				1	1			3	3
Peak-Hour Factor, PHF	0.74		0.74				0.95	0.95			0.92	0.92
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, I ₁		2.0						2.0			2.0	
Extension of Effective Green, e		2.0						2.0			2.0	
Arrival Type, AT		3						3			3	
Unit Extension, UE		3.0						3.0			3.0	
Filtering/Metering, I		1.000						1.000			1.000	
Initial Unmet Demand, Q _b		0.0						0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0
Lane Width		16.0						12.0			12.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b		0						0			0	
Min. Time for Pedestrians, G _p		3.2			3.2			3.2			3.2	
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 20.0	G =	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					

Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		129						265			231	
Lane Group Capacity, c		647						908			908	
v/c Ratio, X		0.20						0.29			0.25	
Total Green Ratio, g/C		0.33						0.50			0.50	
Uniform Delay, d ₁		14.3						8.8			8.6	
Progression Factor, PF		1.000						1.000			1.000	
Delay Calibration, k		0.50						0.50			0.50	
Incremental Delay, d ₂		0.7						0.8			0.7	
Initial Queue Delay, d ₃		0.0						0.0			0.0	
Control Delay		15.0						9.6			9.3	
Lane Group LOS		B						A			A	
Approach Delay	15.0						9.6			9.3		
Approach LOS	B						A			A		
Intersection Delay	10.6			X _C = 0.26			Intersection LOS			B		

HCS+™ DETAILED REPORT

General Information				Site Information			
Analyst	JAG			Intersection	West Ave./ Route 17M		
Agency or Co.	TMA			Area Type	All other areas		
Date Performed	2/9/2009			Jurisdiction	Village of Chester		
Time Period	AM Peak Hour			Analysis Year	Existing Condition		
				Project ID			

Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N ₁	0	1	1	1	1	0	1	1	1	1	1	0	
Lane Group		LT	R	L	TR		L	T	R	L	TR		
Volume, V (vph)	49	32	127	80	24	16	71	83	111	47	136	13	
% Heavy Vehicles, %HV	2	2	2	0	0	0	1	1	1	3	3	3	
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.76	0.76	0.76	0.81	0.81	0.81	0.85	0.85	0.85	
Pretimed (P) or Actuated (A)	A	A	A	A	A	A	A	A	A	A	A	A	
Start-up Lost Time, I ₁		2.0	2.0	2.0	2.0		2.0	2.0	2.0	2.0	2.0		
Extension of Effective Green, e		2.0	2.0	2.0	2.0		2.0	2.0	2.0	2.0	2.0		
Arrival Type, AT		3	3	3	3		3	3	3	3	3		
Unit Extension, UE		3.0	3.0	3.0	3.0		3.0	3.0	3.0	3.0	3.0		
Filtering/Metering, I		1.000	1.000	1.000	1.000		1.000	1.000	1.000	1.000	1.000		
Initial Unmet Demand, Q _b		0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0	
Lane Width		11.0	12.0	14.0	12.0		11.0	11.0	15.0	13.0	13.0		
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N	
Parking Maneuvers, N _m													
Buses Stopping, N _b		0	0	0	0		0	0	0	0	0		
Min. Time for Pedestrians, G _p		12.4			16.2			15.4			14.9		
Phasing	EW Perm	02		03		04		NS Perm	06		07		08
Timing	G = 16.0	G =		G =		G =		G = 29.0	G =		G =		G =
	Y = 5	Y =		Y =		Y =		Y = 5	Y =		Y =		Y =
Duration of Analysis, T = 0.25							Cycle Length, C = 55.0						

Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		88	138	105	53		88	102	137	55	175		
Lane Group Capacity, c		429	461	413	520		620	959	927	694	992		
v/c Ratio, X		0.21	0.30	0.25	0.10		0.14	0.11	0.15	0.08	0.18		
Total Green Ratio, g/C		0.29	0.29	0.29	0.29		0.53	0.53	0.53	0.53	0.53		
Uniform Delay, d ₁		14.7	15.1	14.9	14.2		6.6	6.5	6.7	6.4	6.8		
Progression Factor, PF		1.000	1.000	1.000	1.000		1.000	1.000	1.000	1.000	1.000		
Delay Calibration, k		0.11	0.11	0.11	0.11		0.11	0.11	0.11	0.11	0.11		
Incremental Delay, d ₂		0.2	0.4	0.3	0.1		0.1	0.0	0.1	0.0	0.1		
Initial Queue Delay, d ₃		0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0		
Control Delay		14.9	15.5	15.3	14.3		6.7	6.6	6.7	6.5	6.9		
Lane Group LOS		B	B	B	B		A	A	A	A	A		
Approach Delay		15.3			14.9			6.7			6.8		
Approach LOS		B			B			A			A		
Intersection Delay		10.2			X _c = 0.22			Intersection LOS			B		

HCS+™ DETAILED REPORT

General Information				Site Information			
Analyst	JAG			Intersection	Route 94/Rt 17 SB ramp		
Agency or Co.	TMA			Area Type	All other areas		
Date Performed	2/20/09			Jurisdiction	Village of Chester		
Time Period	PM Peak Hour			Analysis Year	Existing Condition		
				Project ID			

Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N ₁		1	1	1	1					1	1	0	
Lane Group		T	R	L	T					L	TR		
Volume, V (vph)		163	102	107	406					306	1	30	
% Heavy Vehicles, %HV		7	7	4	4					4	4	4	
Peak-Hour Factor, PHF		0.83	0.83	0.90	0.90					0.79	0.79	0.79	
Pretimed (P) or Actuated (A)		A	A	A	A					A	A	A	
Start-up Lost Time, I ₁		2.0	2.0	2.0	2.0					2.0	2.0		
Extension of Effective Green, e		2.0	2.0	2.0	2.0					2.0	2.0		
Arrival Type, AT		3	3	5	5					3	3		
Unit Extension, UE		3.0	3.0	3.0	3.0					3.0	3.0		
Filtering/Metering, I		1.000	1.000	1.000	1.000					1.000	1.000		
Initial Unmet Demand, Q _b		0.0	0.0	0.0	0.0					0.0	0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0	
Lane Width		12.0	13.0	12.0	12.0					11.0	12.0		
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	5	N	
Parking Maneuvers, N _m													
Buses Stopping, N _b		0	0	0	0					0	0		
Min. Time for Pedestrians, G _p		3.2			3.2			3.2			3.2		
Phasing	EW Perm	WB Only	03		04		SB Only	06		07		08	
Timing	G = 40.0	G = 5.0	G =	G =	G = 40.0	G =	G =	G =	G =	G =	G =	G =	
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =	Y =	Y =	Y =	Y =	
Duration of Analysis, T = 0.25							Cycle Length, C = 100.0						

Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		196	123	119	451					387	39	
Lane Group Capacity, c		710	624	611	914					654	608	
v/c Ratio, X		0.28	0.20	0.19	0.49					0.59	0.06	
Total Green Ratio, g/C		0.40	0.40	0.50	0.50					0.40	0.40	
Uniform Delay, d ₁		20.2	19.5	15.3	16.6					23.6	18.5	
Progression Factor, PF		1.000	1.000	0.926	0.333					1.000	1.000	
Delay Calibration, k		0.11	0.11	0.11	0.11					0.18	0.11	
Incremental Delay, d ₂		0.2	0.2	0.2	0.4					1.4	0.0	
Initial Queue Delay, d ₃		0.0	0.0	0.0	0.0					0.0	0.0	
Control Delay		20.4	19.7	14.3	6.0					25.0	18.5	
Lane Group LOS		C	B	B	A					C	B	
Approach Delay	20.2			7.7						24.4		
Approach LOS	C			A						C		
Intersection Delay	16.1			X _c = 0.54			Intersection LOS			B		

HCS+™ DETAILED REPORT

General Information				Site Information			
Analyst	JAG			Intersection	Rt 94/Rt 17 NB ramps		
Agency or Co.	TMA			Area Type	All other areas		
Date Performed	2/20/09			Jurisdiction	Village of Chester		
Time Period	PM Peak Hour			Analysis Year	Existing Condition		
				Project ID			

Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1			1	1	0	1	1			
Lane Group	L	T			T	R		LT	R			
Volume, V (vph)	138	313			275	331	245	1	189			
% Heavy Vehicles, %HV	4	4			2	2	5	5	5			
Peak-Hour Factor, PHF	0.88	0.88			0.96	0.96	0.83	0.83	0.83			
Pretimed (P) or Actuated (A)	A	A			A	A	A	A	A			
Start-up Lost Time, l ₁	2.0	2.0			2.0	2.0		2.0	2.0			
Extension of Effective Green, e	2.0	2.0			2.0	2.0		2.0	2.0			
Arrival Type, AT	5	5			3	3		3	3			
Unit Extension, UE	3.0	3.0			3.0	3.0		3.0	3.0			
Filtering/Metering, I	1.000	1.000			1.000	1.000		1.000	1.000			
Initial Unmet Demand, Q _b	0.0	0.0			0.0	0.0		0.0	0.0			
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0	0	0	0	
Lane Width	12.0	12.0			11.0	13.0		12.0	13.0			
Parking / Grade / Parking	N	0	N	N	6	N	N	5	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0			0	0		0	0			
Min. Time for Pedestrians, G _p	3.2			3.2			3.2			3.2		
Phasing	EW Perm	EB Only	03	04	NB Only	06	07	08				
Timing	G = 50.0	G = 5.0	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 100.0					

Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	157	356			286	345		296	228			
Lane Group Capacity, c	661	1096			874	794		504	465			
v/c Ratio, X	0.24	0.32			0.33	0.43		0.59	0.49			
Total Green Ratio, g/C	0.60	0.60			0.50	0.50		0.30	0.30			
Uniform Delay, d ₁	11.6	9.9			14.9	16.0		29.7	28.7			
Progression Factor, PF	0.935	0.125			1.000	1.000		1.000	1.000			
Delay Calibration, k	0.11	0.11			0.11	0.11		0.18	0.11			
Incremental Delay, d ₂	0.2	0.2			0.2	0.4		1.8	0.8			
Initial Queue Delay, d ₃	0.0	0.0			0.0	0.0		0.0	0.0			
Control Delay	11.0	1.4			15.2	16.4		31.5	29.5			
Lane Group LOS	B	A			B	B		C	C			
Approach Delay	4.4			15.8			30.7					
Approach LOS	A			B			C					
Intersection Delay	17.0			X _C = 0.44			Intersection LOS			B		

HCS+™ DETAILED REPORT

General Information	Site Information
Analyst <i>JAG</i>	Intersection <i>NYS Rt. 94/ NYS Rt. 17M</i>
Agency or Co. <i>TMA</i>	Area Type <i>All other areas</i>
Date Performed <i>2/20/09</i>	Jurisdiction <i>Village of Chester</i>
Time Period <i>PM Peak Hour</i>	Analysis Year <i>Existing Condition</i>
	Project ID

Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _l	1	1	1	1	1	0	2	1	0	1	2	0
Lane Group	L	LT	R	L	TR		L	TR		L	TR	
Volume, V (vph)	201	189	162	47	172	85	277	328	12	86	390	266
% Heavy Vehicles, %HV	3	3	3	5	5	5	6	6	6	3	3	3
Peak-Hour Factor, PHF	0.95	0.95	0.95	0.92	0.92	0.92	0.94	0.94	0.94	0.91	0.91	0.91
Pretimed (P) or Actuated (A)	A	A	A	A	A	A	A	A	A	A	A	A
Start-up Lost Time, l ₁	2.0	2.0	2.0	2.0	2.0		2.0	2.0		2.0	2.0	
Extension of Effective Green, e	2.0	2.0	2.0	2.0	2.0		2.0	2.0		2.0	2.0	
Arrival Type, AT	3	3	3	3	3		3	3		3	3	
Unit Extension, UE	3.0	3.0	3.0	3.0	3.0		3.0	3.0		3.0	3.0	
Filtering/Metering, I	1.000	1.000	1.000	1.000	1.000		1.000	1.000		1.000	1.000	
Initial Unmet Demand, Q _b	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width	12.0	12.0	12.0	12.0	12.0		10.0	10.0		10.0	12.0	
Parking / Grade / Parking	N	-5	N	N	-3	N	N	-5	N	N	5	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0	0	0	0		0	0		0	0	
Min. Time for Pedestrians, G _p	3.2			3.2			3.2			3.2		
Phasing	WB Only	EB Only	03	04	Thru & RT	NB Only	SB Only	08				
Timing	G = 20.0	G = 19.0	G =	G =	G = 14.0	G = 15.0	G = 12.0	G =				
	Y = 4	Y = 4	Y =	Y =	Y = 4	Y = 4	Y = 4	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 100.0					

Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	212	199	171	51	279		295	362		95	721	
Lane Group Capacity, c	341	359	305	349	349		475	563		191	836	
v/c Ratio, X	0.62	0.55	0.56	0.15	0.80		0.62	0.64		0.50	0.86	
Total Green Ratio, g/C	0.19	0.19	0.19	0.20	0.20		0.15	0.33		0.12	0.26	
Uniform Delay, d ₁	37.2	36.7	36.7	33.0	38.1		39.8	28.5		41.2	35.3	
Progression Factor, PF	1.000	1.000	1.000	1.000	1.000		1.000	1.000		1.000	1.000	
Delay Calibration, k	0.20	0.15	0.16	0.11	0.34		0.20	0.22		0.11	0.39	
Incremental Delay, d ₂	3.5	1.9	2.3	0.2	12.4		2.5	2.5		2.0	9.2	
Initial Queue Delay, d ₃	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Control Delay	40.7	38.5	39.1	33.2	50.5		42.3	31.0		43.2	44.5	
Lane Group LOS	D	D	D	C	D		D	C		D	D	
Approach Delay	39.5			47.8			36.1			44.4		
Approach LOS	D			D			D			D		
Intersection Delay	41.4			X _C = 0.71			Intersection LOS			D		

TWO-WAY STOP CONTROL SUMMARY								
General Information				Site Information				
Analyst	JAG			Intersection	Hambletonian and High			
Agency/Co.	TMA			Jurisdiction	Chester			
Date Performed	2/17/2009			Analysis Year	Existing Condition			
Analysis Time Period	PM Peak Hour							
Project Description								
East/West Street: Hambletonian Avenue				North/South Street: High Street				
Intersection Orientation: North-South				Study Period (hrs): 0.25				
Vehicle Volumes and Adjustments								
Major Street	Northbound			Southbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	15	268			173	15		
Peak-Hour Factor, PHF	0.93	0.93	1.00	1.00	0.90	0.90		
Hourly Flow Rate, HFR (veh/h)	16	288	0	0	192	16		
Percent Heavy Vehicles	1	--	--	0	--	--		
Median Type	Undivided							
RT Channelized			0			0		
Lanes	0	1	0	0	1	0		
Configuration	LT						TR	
Upstream Signal		0			0			
Minor Street	Eastbound			Westbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)	36		15					
Peak-Hour Factor, PHF	0.64	1.00	0.64	1.00	1.00	1.00		
Hourly Flow Rate, HFR (veh/h)	56	0	23	0	0	0		
Percent Heavy Vehicles	4	0	4	0	0	0		
Percent Grade (%)	0			0				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	0		
Configuration		LR						
Delay, Queue Length, and Level of Service								
Approach	Northbound	Southbound	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	LT					LR		
v (veh/h)	16						79	
C (m) (veh/h)	1369						573	
v/c	0.01						0.14	
95% queue length	0.04						0.48	
Control Delay (s/veh)	7.7						12.3	
LOS	A					B		
Approach Delay (s/veh)	--	--				12.3		
Approach LOS	--	--				B		

TWO-WAY STOP CONTROL SUMMARY									
General Information				Site Information					
Analyst	JAG			Intersection	Ward and Rt 17M				
Agency/Co.	TMA			Jurisdiction	Goshen				
Date Performed	2/17/2009			Analysis Year	Existing Condition				
Analysis Time Period	PM Peak Hour								
Project Description									
East/West Street: Ward Road				North/South Street: NYS Route 17M					
Intersection Orientation: North-South				Study Period (hrs): 0.25					
Vehicle Volumes and Adjustments									
Major Street		Northbound			Southbound				
Movement	1	2	3	4	5	6			
	L	T	R	L	T	R			
Volume (veh/h)		373	14	54	252				
Peak-Hour Factor, PHF	1.00	0.93	0.93	0.84	0.84	1.00			
Hourly Flow Rate, HFR (veh/h)	0	401	15	64	300	0			
Percent Heavy Vehicles	0	--	--	0	--	--			
Median Type	Undivided								
RT Channelized			0			0			
Lanes	0	1	0	0	1	0			
Configuration			TR	LT					
Upstream Signal		0			0				
Minor Street		Eastbound			Westbound				
Movement	7	8	9	10	11	12			
	L	T	R	L	T	R			
Volume (veh/h)				5		47			
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.81	1.00	0.81			
Hourly Flow Rate, HFR (veh/h)	0	0	0	6	0	58			
Percent Heavy Vehicles	0	0	0	0	0	0			
Percent Grade (%)		0			0				
Flared Approach		N			N				
Storage		0			0				
RT Channelized			0			0			
Lanes	0	0	0	0	0	0			
Configuration					LR				
Delay, Queue Length, and Level of Service									
Approach	Northbound		Southbound		Westbound			Eastbound	
Movement	1	4	7	8	9	10	11	12	
Lane Configuration		LT		LR					
v (veh/h)		64		64					
C (m) (veh/h)		1154		592					
v/c		0.06		0.11					
95% queue length		0.18		0.36					
Control Delay (s/veh)		8.3		11.8					
LOS		A		B					
Approach Delay (s/veh)	--	--	11.8						
Approach LOS	--	--	B						

TWO-WAY STOP CONTROL SUMMARY								
General Information				Site Information				
Analyst	JAG			Intersection	Rt 17M and Main			
Agency/Co.	TMA			Jurisdiction	Chester			
Date Performed	2/17/2009			Analysis Year	Existing Condition			
Analysis Time Period	PM Peak Hour							
Project Description								
East/West Street: NYS Route 17M				North/South Street: Main Street				
Intersection Orientation: East-West				Study Period (hrs): 0.25				
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	19	539			530	122		
Peak-Hour Factor, PHF	0.84	0.84	1.00	1.00	0.95	0.95		
Hourly Flow Rate, HFR (veh/h)	22	641	0	0	557	128		
Percent Heavy Vehicles	3	--	--	0	--	--		
Median Type	Two Way Left Turn Lane							
RT Channelized			0					0
Lanes	1	1	0	0	1	0		
Configuration	L	T						TR
Upstream Signal		0			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)				103		15		
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.60	1.00	0.60		
Hourly Flow Rate, HFR (veh/h)	0	0	0	171	0	24		
Percent Heavy Vehicles	0	0	0	3	0	3		
Percent Grade (%)		0			0			
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	0		
Configuration					LR			
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	L						LR	
v (veh/h)	22						195	
C (m) (veh/h)	904						387	
v/c	0.02						0.50	
95% queue length	0.07						2.74	
Control Delay (s/veh)	9.1						23.4	
LOS	A						C	
Approach Delay (s/veh)	--	--					23.4	
Approach LOS	--	--					C	

HCS+™ DETAILED REPORT

General Information	Site Information
Analyst JAG	Intersection Arcadia and Rt 17M
Agency or Co. TMA	Area Type All other areas
Date Performed 2/17/2009	Jurisdiction Goshen
Time Period PM Peak Hour	Analysis Year Existing Condition
	Project ID

Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0		0				0	1			1	0
Lane Group		LR						LT			TR	
Volume, V (vph)	46		63				63	355			270	54
% Heavy Vehicles, %HV	0		0				1	1			0	0
Peak-Hour Factor, PHF	0.91		0.91				0.88	0.88			0.79	0.79
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, l ₁		2.0						2.0			2.0	
Extension of Effective Green, e		2.0						2.0			2.0	
Arrival Type, AT		3						3			3	
Unit Extension, UE		3.0						3.0			3.0	
Filtering/Metering, I		1.000						1.000			1.000	
Initial Unmet Demand, Q _b		0.0						0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0
Lane Width		16.0						12.0			12.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b		0						0			0	
Min. Time for Pedestrians, G _p		3.2			3.2			3.2			3.2	
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 20.0	G =	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					

Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		120						475			410	
Lane Group Capacity, c		648						835			929	
v/c Ratio, X		0.19						0.57			0.44	
Total Green Ratio, g/C		0.33						0.50			0.50	
Uniform Delay, d ₁		14.2						10.5			9.6	
Progression Factor, PF		1.000						1.000			1.000	
Delay Calibration, k		0.50						0.50			0.50	
Incremental Delay, d ₂		0.6						2.8			1.5	
Initial Queue Delay, d ₃		0.0						0.0			0.0	
Control Delay		14.8						13.3			11.1	
Lane Group LOS		B						B			B	
Approach Delay	14.8						13.3			11.1		
Approach LOS	B						B			B		
Intersection Delay	12.6			X _C = 0.42			Intersection LOS			B		

HCS+™ DETAILED REPORT

General Information						Site Information							
Analyst JAG						Intersection West Ave./ Route 17M							
Agency or Co. TMA						Area Type All other areas							
Date Performed 2/9/2009						Jurisdiction Village of Chester							
Time Period PM Peak Hour						Analysis Year Existing Condition							
Project ID													
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N _i	0	1	1	1	1	0	1	1	1	1	1	0	
Lane Group		LT	R	L	TR		L	T	R	L	TR		
Volume, V (vph)	79	67	117	233	84	39	112	218	235	74	215	21	
% Heavy Vehicles, %HV	3	3	3	1	1	1	0	0	0	0	0	0	
Peak-Hour Factor, PHF	0.81	0.81	0.81	0.88	0.88	0.88	0.94	0.94	0.94	0.98	0.98	0.98	
Pretimed (P) or Actuated (A)	A	A	A	A	A	A	A	A	A	A	A	A	
Start-up Lost Time, l _i		2.0	2.0	2.0	2.0		2.0	2.0	2.0	2.0	2.0		
Extension of Effective Green, e		2.0	2.0	2.0	2.0		2.0	2.0	2.0	2.0	2.0		
Arrival Type, AT		3	3	3	3		3	3	3	3	3		
Unit Extension, UE		3.0	3.0	3.0	3.0		3.0	3.0	3.0	3.0	3.0		
Filtering/Metering, I		1.000	1.000	1.000	1.000		1.000	1.000	1.000	1.000	1.000		
Initial Unmet Demand, Q _b		0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0	
Lane Width		11.0	12.0	14.0	12.0		11.0	11.0	15.0	13.0	13.0		
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N	
Parking Maneuvers, N _m													
Buses Stopping, N _b		0	0	0	0		0	0	0	0	0		
Min. Time for Pedestrians, G _p		12.4			16.2			15.4			14.9		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08					
Timing	G = 16.0	G =	G =	G =	G = 29.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25						Cycle Length, C = 55.0							
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		181	144	265	139		119	232	250	76	240		
Lane Group Capacity, c		392	456	376	521		591	969	937	636	1022		
v/c Ratio, X		0.46	0.32	0.70	0.27		0.20	0.24	0.27	0.12	0.23		
Total Green Ratio, g/C		0.29	0.29	0.29	0.29		0.53	0.53	0.53	0.53	0.53		
Uniform Delay, d ₁		16.0	15.2	17.4	15.0		6.9	7.0	7.2	6.6	7.0		
Progression Factor, PF		1.000	1.000	1.000	1.000		1.000	1.000	1.000	1.000	1.000		
Delay Calibration, k		0.11	0.11	0.27	0.11		0.11	0.11	0.11	0.11	0.11		
Incremental Delay, d ₂		0.9	0.4	5.9	0.3		0.2	0.1	0.2	0.1	0.1		
Initial Queue Delay, d ₃		0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0		
Control Delay		16.8	15.6	23.3	15.3		7.0	7.2	7.3	6.6	7.1		
Lane Group LOS		B	B	C	B		A	A	A	A	A		
Approach Delay		16.3			20.5			7.2			7.0		
Approach LOS		B			C			A			A		
Intersection Delay		12.2			X _C = 0.42			Intersection LOS			B		

HCS+™ DETAILED REPORT

General Information				Site Information			
Analyst	JAG			Intersection	Route 94/Rt 17 SB ramp		
Agency or Co.	TMA			Area Type	All other areas		
Date Performed	2/20/09			Jurisdiction	Village of Chester		
Time Period	Saturday Peak Hour			Analysis Year	Existing Condition		
				Project ID			

Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N ₁		1	1	1	1					1	1	0	
Lane Group		T	R	L	T					L	TR		
Volume, V (vph)		124	89	138	195					266	1	42	
% Heavy Vehicles, %HV		2	2	1	1					2	2	2	
Peak-Hour Factor, PHF		0.86	0.86	0.91	0.91					0.89	0.89	0.89	
Pretimed (P) or Actuated (A)		A	A	A	A					A	A	A	
Start-up Lost Time, I ₁		2.0	2.0	2.0	2.0					2.0	2.0		
Extension of Effective Green, e		2.0	2.0	2.0	2.0					2.0	2.0		
Arrival Type, AT		3	3	5	5					3	3		
Unit Extension, UE		3.0	3.0	3.0	3.0					3.0	3.0		
Filtering/Metering, I		1.000	1.000	1.000	1.000					1.000	1.000		
Initial Unmet Demand, Q _b		0.0	0.0	0.0	0.0					0.0	0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0	
Lane Width		12.0	13.0	12.0	12.0					11.0	12.0		
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	5	N	
Parking Maneuvers, N _m													
Buses Stopping, N _b		0	0	0	0					0	0		
Min. Time for Pedestrians, G _p		3.2			3.2			3.2			3.2		
Phasing	EW Perm	WB Only	03		04		SB Only	06		07		08	
Timing	G = 40.0	G = 5.0	G =	G =	G = 40.0	G =	G =	G =					
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 100.0						

Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		144	103	152	214					299	48	
Lane Group Capacity, c		745	654	677	941					667	620	
v/c Ratio, X		0.19	0.16	0.22	0.23					0.45	0.08	
Total Green Ratio, g/C		0.40	0.40	0.50	0.50					0.40	0.40	
Uniform Delay, d ₁		19.5	19.2	14.4	14.1					21.9	18.6	
Progression Factor, PF		1.000	1.000	0.926	0.333					1.000	1.000	
Delay Calibration, k		0.11	0.11	0.11	0.11					0.11	0.11	
Incremental Delay, d ₂		0.1	0.1	0.2	0.1					0.5	0.1	
Initial Queue Delay, d ₃		0.0	0.0	0.0	0.0					0.0	0.0	
Control Delay		19.6	19.3	13.5	4.8					22.4	18.6	
Lane Group LOS		B	B	B	A					C	B	
Approach Delay	19.5			8.4						21.9		
Approach LOS	B			A						C		
Intersection Delay	16.1			X _c = 0.33			Intersection LOS			B		

HCS+™ DETAILED REPORT

General Information				Site Information			
Analyst	JAG			Intersection	Rt 94/Rt 17 NB ramps		
Agency or Co.	TMA			Area Type	All other areas		
Date Performed	2/20/09			Jurisdiction	Village of Chester		
Time Period	Saturday Peak Hour			Analysis Year	Existing Condition		
				Project ID			

Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1			1	1	0	1	1			
Lane Group	L	T			T	R		LT	R			
Volume, V (vph)	34	351			239	321	100	0	152			
% Heavy Vehicles, %HV	1	1			1	1	2	2	2			
Peak-Hour Factor, PHF	0.97	0.97			0.85	0.85	0.94	0.94	0.94			
Pretimed (P) or Actuated (A)	A	A			A	A	A	A	A			
Start-up Lost Time, I ₁	2.0	2.0			2.0	2.0		2.0	2.0			
Extension of Effective Green, e	2.0	2.0			2.0	2.0		2.0	2.0			
Arrival Type, AT	5	5			3	3		3	3			
Unit Extension, UE	3.0	3.0			3.0	3.0		3.0	3.0			
Filtering/Metering, I	1.000	1.000			1.000	1.000		1.000	1.000			
Initial Unmet Demand, Q _b	0.0	0.0			0.0	0.0		0.0	0.0			
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0	0	0	0	
Lane Width	12.0	12.0			11.0	13.0		12.0	13.0			
Parking / Grade / Parking	N	0	N	N	6	N	N	5	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0			0	0		0	0			
Min. Time for Pedestrians, G _p	3.2			3.2			3.2			3.2		
Phasing	EW Perm	EB Only	03	04	NB Only	06	07	08				
Timing	G = 50.0	G = 5.0	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 100.0					

Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	35	362			281	378		106	162			
Lane Group Capacity, c	685	1129			882	802		519	479			
v/c Ratio, X	0.05	0.32			0.32	0.47		0.20	0.34			
Total Green Ratio, g/C	0.60	0.60			0.50	0.50		0.30	0.30			
Uniform Delay, d ₁	10.1	9.9			14.9	16.4		26.1	27.3			
Progression Factor, PF	0.935	0.125			1.000	1.000		1.000	1.000			
Delay Calibration, k	0.11	0.11			0.11	0.11		0.11	0.11			
Incremental Delay, d ₂	0.0	0.2			0.2	0.4		0.2	0.4			
Initial Queue Delay, d ₃	0.0	0.0			0.0	0.0		0.0	0.0			
Control Delay	9.5	1.4			15.1	16.8		26.3	27.7			
Lane Group LOS	A	A			B	B		C	C			
Approach Delay	2.1			16.1			27.1					
Approach LOS	A			B			C					
Intersection Delay	14.1			X _C = 0.37			Intersection LOS			B		

HCS+™ DETAILED REPORT

General Information	Site Information
Analyst <i>JAG</i>	Intersection <i>NYS Rt. 94/ NYS Rt. 17M</i>
Agency or Co. <i>TMA</i>	Area Type <i>All other areas</i>
Date Performed <i>2/20/09</i>	Jurisdiction <i>Village of Chester</i>
Time Period <i>Saturday Peak Hour</i>	Analysis Year <i>Existing Condition</i>
	Project ID

Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _l	1	1	1	1	1	0	2	1	0	1	2	0
Lane Group	L	LT	R	L	TR		L	TR		L	TR	
Volume, V (vph)	245	148	252	61	155	110	293	308	26	117	256	175
% Heavy Vehicles, %HV	3	3	3	3	3	3	1	1	1	2	2	2
Peak-Hour Factor, PHF	0.87	0.87	0.87	0.86	0.86	0.86	0.94	0.94	0.94	0.94	0.94	0.94
Pretimed (P) or Actuated (A)	A	A	A	A	A	A	A	A	A	A	A	A
Start-up Lost Time, l ₁	2.0	2.0	2.0	2.0	2.0		2.0	2.0		2.0	2.0	
Extension of Effective Green, e	2.0	2.0	2.0	2.0	2.0		2.0	2.0		2.0	2.0	
Arrival Type, AT	3	3	3	3	3		3	3		3	3	
Unit Extension, UE	3.0	3.0	3.0	3.0	3.0		3.0	3.0		3.0	3.0	
Filtering/Metering, I	1.000	1.000	1.000	1.000	1.000		1.000	1.000		1.000	1.000	
Initial Unmet Demand, Q _b	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width	12.0	12.0	12.0	12.0	12.0		10.0	10.0		10.0	12.0	
Parking / Grade / Parking	N	-5	N	N	-3	N	N	-5	N	N	5	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0	0	0	0		0	0		0	0	
Min. Time for Pedestrians, G _p	3.2			3.2			3.2			3.2		
Phasing	WB Only	EB Only	03	04	Thru & RT	NB Only	SB Only	08				
Timing	G = 21.0	G = 23.0	G =	G =	G = 12.0	G = 12.0	G = 12.0	G =				
	Y = 4	Y = 4	Y =	Y =	Y = 4	Y = 4	Y = 4	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 100.0					

Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	282	170	290	71	308		312	356		124	458	
Lane Group Capacity, c	413	435	370	374	369		398	498		193	779	
v/c Ratio, X	0.68	0.39	0.78	0.19	0.83		0.78	0.71		0.64	0.59	
Total Green Ratio, g/C	0.23	0.23	0.23	0.21	0.21		0.12	0.28		0.12	0.24	
Uniform Delay, d ₁	35.2	32.6	36.2	32.5	37.8		42.7	32.4		42.0	33.6	
Progression Factor, PF	1.000	1.000	1.000	1.000	1.000		1.000	1.000		1.000	1.000	
Delay Calibration, k	0.25	0.11	0.33	0.11	0.37		0.33	0.28		0.22	0.18	
Incremental Delay, d ₂	4.6	0.6	10.5	0.2	15.2		9.9	4.8		7.1	1.2	
Initial Queue Delay, d ₃	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Control Delay	39.8	33.2	46.7	32.7	53.0		52.6	37.3		49.1	34.8	
Lane Group LOS	D	C	D	C	D		D	D		D	C	
Approach Delay	41.0			49.2			44.4			37.8		
Approach LOS	D			D			D			D		
Intersection Delay	42.5			X _C = 0.75			Intersection LOS			D		

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	JAG			Intersection	Hambletonian and High		
Agency/Co.	TMA			Jurisdiction	Chester		
Date Performed	2/17/2009			Analysis Year	Existing Condition		
Analysis Time Period	Saturday Peak Hour						
Project Description							
East/West Street: Hambletonian Avenue				North/South Street: High Street			
Intersection Orientation: North-South				Study Period (hrs): 0.25			
Vehicle Volumes and Adjustments							
Major Street		Northbound			Southbound		
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)	17	244			265	28	
Peak-Hour Factor, PHF	0.91	0.91	1.00	1.00	0.88	0.88	
Hourly Flow Rate, HFR (veh/h)	18	268	0	0	301	31	
Percent Heavy Vehicles	1	--	--	0	--	--	
Median Type	Undivided						
RT Channelized			0			0	
Lanes	0	1	0	0	1	0	
Configuration	LT					TR	
Upstream Signal		0			0		
Minor Street		Eastbound			Westbound		
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)	40		22				
Peak-Hour Factor, PHF	0.70	1.00	0.70	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	57	0	31	0	0	0	
Percent Heavy Vehicles	4	0	4	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0			0	
Lanes	0	0	0	0	0	0	
Configuration		LR					
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration	LT						LR
v (veh/h)	18						88
C (m) (veh/h)	1233						511
v/c	0.01						0.17
95% queue length	0.04						0.62
Control Delay (s/veh)	8.0						13.5
LOS	A						B
Approach Delay (s/veh)	--	--					13.5
Approach LOS	--	--					B

TWO-WAY STOP CONTROL SUMMARY								
General Information				Site Information				
Analyst	JAG			Intersection	Ward and Rt 17M			
Agency/Co.	TMA			Jurisdiction	Goshen			
Date Performed	2/17/2009			Analysis Year	Existing Condition			
Analysis Time Period	Saturday Peak Hour							
Project Description								
East/West Street: Ward Road				North/South Street: NYS Route 17M				
Intersection Orientation: North-South				Study Period (hrs): 0.25				
Vehicle Volumes and Adjustments								
Major Street		Northbound			Southbound			
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)		292	7	28	217			
Peak-Hour Factor, PHF	1.00	0.91	0.91	0.86	0.86	1.00		
Hourly Flow Rate, HFR (veh/h)	0	320	7	32	252	0		
Percent Heavy Vehicles	0	--	--	0	--	--		
Median Type	Undivided							
RT Channelized			0					0
Lanes	0	1	0	0	1	0		
Configuration			TR	LT				
Upstream Signal		0			0			
Minor Street		Eastbound			Westbound			
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)				2		13		
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.63	1.00	0.63		
Hourly Flow Rate, HFR (veh/h)	0	0	0	3	0	20		
Percent Heavy Vehicles	0	0	0	0	0	0		
Percent Grade (%)		0			0			
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	0		
Configuration					LR			
Delay, Queue Length, and Level of Service								
Approach	Northbound	Southbound	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration		LT		LR				
v (veh/h)		32		23				
C (m) (veh/h)		1244		664				
v/c		0.03		0.03				
95% queue length		0.08		0.11				
Control Delay (s/veh)		8.0		10.6				
LOS		A		B				
Approach Delay (s/veh)	--	--	10.6					
Approach LOS	--	--	B					

TWO-WAY STOP CONTROL SUMMARY								
General Information				Site Information				
Analyst	JAG			Intersection	Rt 17M and Main			
Agency/Co.	TMA			Jurisdiction	Chester			
Date Performed	2/17/2009			Analysis Year	Existing Condition			
Analysis Time Period	Saturday Peak Hour							
Project Description								
East/West Street: NYS Route 17M				North/South Street: Main Street				
Intersection Orientation: East-West				Study Period (hrs): 0.25				
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	16	377			514	88		
Peak-Hour Factor, PHF	0.95	0.95	1.00	1.00	0.98	0.98		
Hourly Flow Rate, HFR (veh/h)	16	396	0	0	524	89		
Percent Heavy Vehicles	3	--	--	0	--	--		
Median Type	Two Way Left Turn Lane							
RT Channelized			0			0		
Lanes	1	1	0	0	1	0		
Configuration	L	T				TR		
Upstream Signal		0			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)				56		34		
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.98	1.00	0.98		
Hourly Flow Rate, HFR (veh/h)	0	0	0	57	0	34		
Percent Heavy Vehicles	0	0	0	2	0	2		
Percent Grade (%)		0			0			
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	0		
Configuration					LR			
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	L						LR	
v (veh/h)	16						91	
C (m) (veh/h)	961						484	
v/c	0.02						0.19	
95% queue length	0.05						0.69	
Control Delay (s/veh)	8.8						14.2	
LOS	A						B	
Approach Delay (s/veh)	--	--					14.2	
Approach LOS	--	--					B	

HCS+™ DETAILED REPORT

General Information		Site Information	
Analyst	JAG	Intersection	Arcadia and Rt 17M
Agency or Co.	TMA	Area Type	All other areas
Date Performed	2/17/2009	Jurisdiction	Goshen
Time Period	Saturday Peak Hour	Analysis Year	Existing Condition
		Project ID	

Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _l	0		0				0	1			1	0
Lane Group		LR						LT			TR	
Volume, V (vph)	39		40				46	249			189	47
% Heavy Vehicles, %HV	0		0				0	0			0	0
Peak-Hour Factor, PHF	0.94		0.94				0.96	0.96			0.80	0.80
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, l ₁		2.0						2.0			2.0	
Extension of Effective Green, e		2.0						2.0			2.0	
Arrival Type, AT		3						3			3	
Unit Extension, UE		3.0						3.0			3.0	
Filtering/Metering, I		1.000						1.000			1.000	
Initial Unmet Demand, Q _b		0.0						0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0
Lane Width		16.0						12.0			12.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b		0						0			0	
Min. Time for Pedestrians, G _p		3.2			3.2			3.2			3.2	
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 20.0	G =	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					

Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		84						307			295	
Lane Group Capacity, c		652						871			925	
v/c Ratio, X		0.13						0.35			0.32	
Total Green Ratio, g/C		0.33						0.50			0.50	
Uniform Delay, d ₁		13.9						9.1			8.9	
Progression Factor, PF		1.000						1.000			1.000	
Delay Calibration, k		0.50						0.50			0.50	
Incremental Delay, d ₂		0.4						1.1			0.9	
Initial Queue Delay, d ₃		0.0						0.0			0.0	
Control Delay		14.3						10.2			9.8	
Lane Group LOS		B						B			A	
Approach Delay		14.3						10.2			9.8	
Approach LOS		B						B			A	
Intersection Delay		10.6			X _C = 0.26			Intersection LOS			B	

HCS+™ DETAILED REPORT

General Information				Site Information			
Analyst	JAG			Intersection	West Ave./ Route 17M		
Agency or Co.	TMA			Area Type	All other areas		
Date Performed	2/9/2009			Jurisdiction	Village of Chester		
Time Period	Saturday Peak Hour			Analysis Year	Existing Condition		
				Project ID			

Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N _i	0	1	1	1	1	0	1	1	1	1	1	0	
Lane Group		LT	R	L	TR		L	T	R	L	TR		
Volume, V (vph)	36	118	87	257	111	27	74	217	358	90	146	33	
% Heavy Vehicles, %HV	3	3	3	1	1	1	2	2	2	2	2	2	
Peak-Hour Factor, PHF	0.81	0.81	0.81	0.95	0.95	0.95	0.95	0.95	0.95	0.92	0.92	0.92	
Pretimed (P) or Actuated (A)	A	A	A	A	A	A	A	A	A	A	A	A	
Start-up Lost Time, l _i		2.0	2.0	2.0	2.0		2.0	2.0	2.0	2.0	2.0		
Extension of Effective Green, e		2.0	2.0	2.0	2.0		2.0	2.0	2.0	2.0	2.0		
Arrival Type, AT		3	3	3	3		3	3	3	3	3		
Unit Extension, UE		3.0	3.0	3.0	3.0		3.0	3.0	3.0	3.0	3.0		
Filtering/Metering, I		1.000	1.000	1.000	1.000		1.000	1.000	1.000	1.000	1.000		
Initial Unmet Demand, Q _b		0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0	
Lane Width		11.0	12.0	14.0	12.0		11.0	11.0	15.0	13.0	13.0		
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N	
Parking Maneuvers, N _m													
Buses Stopping, N _b		0	0	0	0		0	0	0	0	0		
Min. Time for Pedestrians, G _p		12.4			16.2			15.4			14.9		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08					
Timing	G = 16.0	G =	G =	G =	G = 29.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 55.0						

Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		190	107	271	145		78	228	377	98	195		
Lane Group Capacity, c		468	456	372	531		603	950	919	625	987		
v/c Ratio, X		0.41	0.23	0.73	0.27		0.13	0.24	0.41	0.16	0.20		
Total Green Ratio, g/C		0.29	0.29	0.29	0.29		0.53	0.53	0.53	0.53	0.53		
Uniform Delay, d ₁		15.7	14.8	17.5	15.0		6.6	7.0	7.8	6.7	6.9		
Progression Factor, PF		1.000	1.000	1.000	1.000		1.000	1.000	1.000	1.000	1.000		
Delay Calibration, k		0.11	0.11	0.29	0.11		0.11	0.11	0.11	0.11	0.11		
Incremental Delay, d ₂		0.6	0.3	7.1	0.3		0.1	0.1	0.3	0.1	0.1		
Initial Queue Delay, d ₃		0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0		
Control Delay		16.3	15.1	24.6	15.3		6.7	7.2	8.1	6.8	7.0		
Lane Group LOS		B	B	C	B		A	A	A	A	A		
Approach Delay		15.8			21.4			7.7			6.9		
Approach LOS		B			C			A			A		
Intersection Delay		12.3			X _C = 0.52			Intersection LOS			B		

HCS+™ DETAILED REPORT

General Information				Site Information			
Analyst	JAG	Intersection	Route 94/Rt 17 SB ramp				
Agency or Co.	TMA	Area Type	All other areas				
Date Performed	2/20/09	Jurisdiction	Village of Chester				
Time Period	AM Peak Hour	Analysis Year	No Build Condition				
		Project ID					

Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N _l		1	1	1	1					1	1	0	
Lane Group		T	R	L	T					L	TR		
Volume, V (vph)		171	288	269	216					269	1	406	
% Heavy Vehicles, %HV		8	8	5	5					5	5	5	
Peak-Hour Factor, PHF		0.94	0.94	0.78	0.78					0.71	0.71	0.71	
Pretimed (P) or Actuated (A)		A	A	A	A					A	A	A	
Start-up Lost Time, l ₁		2.0	2.0	2.0	2.0					2.0	2.0		
Extension of Effective Green, e		2.0	2.0	2.0	2.0					2.0	2.0		
Arrival Type, AT		3	3	5	5					3	3		
Unit Extension, UE		3.0	3.0	3.0	3.0					3.0	3.0		
Filtering/Metering, I		1.000	1.000	1.000	1.000					1.000	1.000		
Initial Unmet Demand, Q _b		0.0	0.0	0.0	0.0					0.0	0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0	
Lane Width		12.0	13.0	12.0	12.0					11.0	12.0		
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	5	N	
Parking Maneuvers, N _m													
Buses Stopping, N _b		0	0	0	0					0	0		
Min. Time for Pedestrians, G _p		3.2			3.2			3.2			3.2		
Phasing	EW Perm	WB Only	03		04		SB Only	06		07		08	
Timing	G = 38.0	G = 5.0	G =		G =		G = 42.0	G =		G =		G =	
	Y = 5	Y = 5	Y =		Y =		Y = 5	Y =		Y =		Y =	
Duration of Analysis, T = 0.25							Cycle Length, C = 100.0						

Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		182	306	345	277					379	573	
Lane Group Capacity, c		668	587	591	869					680	630	
v/c Ratio, X		0.27	0.52	0.58	0.32					0.56	0.91	
Total Green Ratio, g/C		0.38	0.38	0.48	0.48					0.42	0.42	
Uniform Delay, d ₁		21.4	24.0	20.7	16.0					22.0	27.2	
Progression Factor, PF		1.000	1.000	0.926	0.385					1.000	1.000	
Delay Calibration, k		0.11	0.13	0.18	0.11					0.15	0.43	
Incremental Delay, d ₂		0.2	0.8	1.5	0.2					1.0	17.3	
Initial Queue Delay, d ₃		0.0	0.0	0.0	0.0					0.0	0.0	
Control Delay		21.7	24.8	20.7	6.4					23.0	44.5	
Lane Group LOS		C	C	C	A					C	D	
Approach Delay	23.6			14.3						36.0		
Approach LOS	C			B						D		
Intersection Delay	26.5			X _C = 0.77			Intersection LOS			C		

HCS+™ DETAILED REPORT

General Information	Site Information
Analyst <i>JAG</i>	Intersection <i>Rt 94/Rt 17 NB ramps</i>
Agency or Co. <i>TMA</i>	Area Type <i>All other areas</i>
Date Performed <i>2/20/09</i>	Jurisdiction <i>Village of Chester</i>
Time Period <i>AM Peak Hour</i>	Analysis Year <i>No Build Condition</i>
	Project ID

Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _l	1	1			1	1	0	1	1			
Lane Group	L	T			T	R		LT	R			
Volume, V (vph)	112	371			386	341	163	1	94			
% Heavy Vehicles, %HV	5	5			2	2	8	8	8			
Peak-Hour Factor, PHF	0.91	0.91			0.94	0.94	0.77	0.77	0.77			
Pretimed (P) or Actuated (A)	A	A			A	A	A	A	A			
Start-up Lost Time, l ₁	2.0	2.0			2.0	2.0		2.0	2.0			
Extension of Effective Green, e	2.0	2.0			2.0	2.0		2.0	2.0			
Arrival Type, AT	5	5			3	3		3	3			
Unit Extension, UE	3.0	3.0			3.0	3.0		3.0	3.0			
Filtering/Metering, I	1.000	1.000			1.000	1.000		1.000	1.000			
Initial Unmet Demand, Q _b	0.0	0.0			0.0	0.0		0.0	0.0			
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0	0	0	0	
Lane Width	12.0	12.0			11.0	13.0		12.0	13.0			
Parking / Grade / Parking	N	0	N	N	6	N	N	5	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0			0	0		0	0			
Min. Time for Pedestrians, G _p	3.2			3.2			3.2			3.2		
Phasing	EW Perm	EB Only	03	04	NB Only	06	07	08				
Timing	G = 50.0	G = 5.0	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 100.0					

Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	123	408			411	363		213	122			
Lane Group Capacity, c	551	1086			874	794		490	452			
v/c Ratio, X	0.22	0.38			0.47	0.46		0.43	0.27			
Total Green Ratio, g/C	0.60	0.60			0.50	0.50		0.30	0.30			
Uniform Delay, d ₁	14.3	10.3			16.3	16.2		28.2	26.7			
Progression Factor, PF	0.935	0.125			1.000	1.000		1.000	1.000			
Delay Calibration, k	0.11	0.11			0.11	0.11		0.11	0.11			
Incremental Delay, d ₂	0.2	0.2			0.4	0.4		0.6	0.3			
Initial Queue Delay, d ₃	0.0	0.0			0.0	0.0		0.0	0.0			
Control Delay	13.5	1.5			16.7	16.6		28.8	27.0			
Lane Group LOS	B	A			B	B		C	C			
Approach Delay	4.3			16.7			28.1					
Approach LOS	A			B			C					
Intersection Delay	15.0			X _C = 0.41			Intersection LOS			B		

HCS+™ DETAILED REPORT

General Information	Site Information
Analyst <i>JAG</i>	Intersection <i>NYS Rt. 94/ NYS Rt. 17M</i>
Agency or Co. <i>TMA</i>	Area Type <i>All other areas</i>
Date Performed <i>2/20/09</i>	Jurisdiction <i>Village of Chester</i>
Time Period <i>AM Peak Hour</i>	Analysis Year <i>No Build Condition</i>
	Project ID

Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _l	1	1	1	1	1	0	2	1	0	1	2	0
Lane Group	L	T	R	L	TR		L	TR		L	TR	
Volume, V (vph)	91	108	196	40	183	65	307	248	8	42	186	212
% Heavy Vehicles, %HV	13	13	13	6	6	6	6	6	6	7	7	7
Peak-Hour Factor, PHF	0.83	0.83	0.83	0.86	0.86	0.86	0.92	0.92	0.92	0.86	0.86	0.86
Pretimed (P) or Actuated (A)	A	A	A	A	A	A	A	A	A	A	A	A
Start-up Lost Time, l ₁	2.0	2.0	2.0	2.0	2.0		2.0	2.0		2.0	2.0	
Extension of Effective Green, e	2.0	2.0	2.0	2.0	2.0		2.0	2.0		2.0	2.0	
Arrival Type, AT	3	3	3	3	3		3	3		3	3	
Unit Extension, UE	3.0	3.0	3.0	3.0	3.0		3.0	3.0		3.0	3.0	
Filtering/Metering, I	1.000	1.000	1.000	1.000	1.000		1.000	1.000		1.000	1.000	
Initial Unmet Demand, Q _b	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width	12.0	12.0	12.0	12.0	12.0		10.0	10.0		10.0	12.0	
Parking / Grade / Parking	N	-5	N	N	-3	N	N	-5	N	N	5	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0	0	0	0		0	0		0	0	
Min. Time for Pedestrians, G _p	3.2			3.2			3.2			3.2		
Phasing	EW Perm	EB Only	03	04	NS Perm	NB Only	SB Only	08				
Timing	G = 27.0	G = 12.0	G =	G =	G = 16.0	G = 14.0	G = 11.0	G =				
	Y = 4	Y = 4	Y =	Y =	Y = 4	Y = 4	Y = 4	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 100.0					

Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	110	130	236	47	289		334	279		49	463	
Lane Group Capacity, c	433	741	630	326	472		443	580		244	819	
v/c Ratio, X	0.25	0.18	0.37	0.14	0.61		0.75	0.48		0.20	0.57	
Total Green Ratio, g/C	0.43	0.43	0.43	0.27	0.27		0.14	0.34		0.27	0.27	
Uniform Delay, d ₁	25.2	17.6	19.4	27.7	31.9		41.3	26.0		34.7	31.4	
Progression Factor, PF	1.000	1.000	1.000	1.000	1.000		1.000	1.000		1.000	1.000	
Delay Calibration, k	0.11	0.11	0.11	0.11	0.20		0.31	0.11		0.11	0.16	
Incremental Delay, d ₂	0.3	0.1	0.4	0.2	2.3		7.2	0.6		0.4	0.9	
Initial Queue Delay, d ₃	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Control Delay	25.5	17.7	19.7	27.9	34.3		48.5	26.7		35.1	32.4	
Lane Group LOS	C	B	B	C	C		D	C		D	C	
Approach Delay	20.5			33.4			38.6			32.6		
Approach LOS	C			C			D			C		
Intersection Delay	31.7			X _C = 0.49			Intersection LOS			C		

TWO-WAY STOP CONTROL SUMMARY								
General Information				Site Information				
Analyst	JAG			Intersection	Hambletonian and High			
Agency/Co.	TMA			Jurisdiction	Chester			
Date Performed	2/17/2009			Analysis Year	No Build Condition			
Analysis Time Period	AM Peak Hour							
Project Description								
East/West Street: Hambletonian Avenue				North/South Street: High Street				
Intersection Orientation: North-South				Study Period (hrs): 0.25				
Vehicle Volumes and Adjustments								
Major Street	Northbound			Southbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	111	198			345	123		
Peak-Hour Factor, PHF	0.95	0.95	1.00	1.00	0.89	0.89		
Hourly Flow Rate, HFR (veh/h)	116	208	0	0	387	138		
Percent Heavy Vehicles	10	--	--	0	--	--		
Median Type	Undivided							
RT Channelized			0			0		
Lanes	0	1	0	0	1	0		
Configuration	LT						TR	
Upstream Signal		0			0			
Minor Street	Eastbound			Westbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)	68		106					
Peak-Hour Factor, PHF	0.60	1.00	0.60	1.00	1.00	1.00		
Hourly Flow Rate, HFR (veh/h)	113	0	176	0	0	0		
Percent Heavy Vehicles	12	0	12	0	0	0		
Percent Grade (%)	0			0				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	0		
Configuration		LR						
Delay, Queue Length, and Level of Service								
Approach	Northbound	Southbound	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	LT						LR	
v (veh/h)	116						289	
C (m) (veh/h)	1002						396	
v/c	0.12						0.73	
95% queue length	0.39						5.69	
Control Delay (s/veh)	9.1						35.0	
LOS	A						E	
Approach Delay (s/veh)	--	--					35.0	
Approach LOS	--	--					E	

TWO-WAY STOP CONTROL SUMMARY								
General Information				Site Information				
Analyst	JAG			Intersection	Ward and Rt 17M			
Agency/Co.	TMA			Jurisdiction	Goshen			
Date Performed	2/17/2009			Analysis Year	No Build Condition			
Analysis Time Period	AM Peak Hour							
Project Description								
East/West Street: Ward Road				North/South Street: NYS Route 17M				
Intersection Orientation: North-South				Study Period (hrs): 0.25				
Vehicle Volumes and Adjustments								
Major Street		Northbound			Southbound			
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)		234	3	25	244			
Peak-Hour Factor, PHF	1.00	0.87	0.87	0.92	0.92	1.00		
Hourly Flow Rate, HFR (veh/h)	0	268	3	27	265	0		
Percent Heavy Vehicles	0	--	--	1	--	--		
Median Type	Undivided							
RT Channelized			0			0		
Lanes	0	1	0	0	1	0		
Configuration			TR	LT				
Upstream Signal		0			0			
Minor Street		Eastbound			Westbound			
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)				3		69		
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.89	1.00	0.89		
Hourly Flow Rate, HFR (veh/h)	0	0	0	3	0	77		
Percent Heavy Vehicles	0	0	0	1	0	1		
Percent Grade (%)		0			0			
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	0		
Configuration					LR			
Delay, Queue Length, and Level of Service								
Approach	Northbound	Southbound	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration		LT		LR				
v (veh/h)		27		80				
C (m) (veh/h)		1298		752				
v/c		0.02		0.11				
95% queue length		0.06		0.36				
Control Delay (s/veh)		7.8		10.4				
LOS		A		B				
Approach Delay (s/veh)	--	--	10.4					
Approach LOS	--	--	B					

TWO-WAY STOP CONTROL SUMMARY								
General Information				Site Information				
Analyst	JAG			Intersection	Rt 17M and Main			
Agency/Co.	TMA			Jurisdiction	Chester			
Date Performed	2/17/2009			Analysis Year	No Build Condition			
Analysis Time Period	AM Peak Hour							
Project Description								
East/West Street: NYS Route 17M				North/South Street: Main Street				
Intersection Orientation: East-West				Study Period (hrs): 0.25				
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	5	308			485	82		
Peak-Hour Factor, PHF	0.94	0.94	1.00	1.00	0.85	0.85		
Hourly Flow Rate, HFR (veh/h)	5	327	0	0	570	96		
Percent Heavy Vehicles	7	--	--	0	--	--		
Median Type	Two Way Left Turn Lane							
RT Channelized			0			0		
Lanes	1	1	0	0	1	0		
Configuration	L	T				TR		
Upstream Signal		0			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)				105		13		
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.91	1.00	0.91		
Hourly Flow Rate, HFR (veh/h)	0	0	0	115	0	14		
Percent Heavy Vehicles	0	0	0	1	0	1		
Percent Grade (%)	0			0				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	0		
Configuration					LR			
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	L						LR	
v (veh/h)	5						129	
C (m) (veh/h)	900						472	
v/c	0.01						0.27	
95% queue length	0.02						1.10	
Control Delay (s/veh)	9.0						15.5	
LOS	A						C	
Approach Delay (s/veh)	--	--					15.5	
Approach LOS	--	--					C	

HCS+™ DETAILED REPORT

General Information		Site Information	
Analyst	JAG	Intersection	Arcadia and Rt 17M
Agency or Co.	TMA	Area Type	All other areas
Date Performed	2/17/2009	Jurisdiction	Goshen
Time Period	AM Peak Hour	Analysis Year	No Build Condition
		Project ID	

Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _l	0		0				0	1			1	0
Lane Group		LR						LT			TR	
Volume, V (vph)	56		61				29	268			229	29
% Heavy Vehicles, %HV	1		1				1	1			3	3
Peak-Hour Factor, PHF	0.74		0.74				0.95	0.95			0.92	0.92
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, l ₁		2.0						2.0			2.0	
Extension of Effective Green, e		2.0						2.0			2.0	
Arrival Type, AT		3						3			3	
Unit Extension, UE		3.0						3.0			3.0	
Filtering/Metering, I		1.000						1.000			1.000	
Initial Unmet Demand, Q _b		0.0						0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0
Lane Width		16.0						12.0			12.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b		0						0			0	
Min. Time for Pedestrians, G _p		3.2			3.2			3.2			3.2	
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 20.0	G =	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					

Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		158						313			281	
Lane Group Capacity, c		645						897			908	
v/c Ratio, X		0.24						0.35			0.31	
Total Green Ratio, g/C		0.33						0.50			0.50	
Uniform Delay, d ₁		14.5						9.1			8.9	
Progression Factor, PF		1.000						1.000			1.000	
Delay Calibration, k		0.50						0.50			0.50	
Incremental Delay, d ₂		0.9						1.1			0.9	
Initial Queue Delay, d ₃		0.0						0.0			0.0	
Control Delay		15.4						10.2			9.8	
Lane Group LOS		B						B			A	
Approach Delay		15.4						10.2			9.8	
Approach LOS		B						B			A	
Intersection Delay		11.1			X _C = 0.31			Intersection LOS			B	

HCS+™ DETAILED REPORT

General Information				Site Information			
Analyst	JAG			Intersection	West Ave./ Route 17M		
Agency or Co.	TMA			Area Type	All other areas		
Date Performed	2/9/2009			Jurisdiction	Village of Chester		
Time Period	AM Peak Hour			Analysis Year	No Build Condition		
				Project ID			

Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N _i	0	1	1	1	1	0	1	1	1	1	1	0	
Lane Group		LT	R	L	TR		L	T	R	L	TR		
Volume, V (vph)	60	101	105	228	91	38	81	224	252	103	168	57	
% Heavy Vehicles, %HV	2	2	2	0	0	0	1	1	1	3	3	3	
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.76	0.76	0.76	0.81	0.81	0.81	0.85	0.85	0.85	
Pretimed (P) or Actuated (A)	A	A	A	A	A	A	A	A	A	A	A	A	
Start-up Lost Time, I ₁		2.0	2.0	2.0	2.0		2.0	2.0	2.0	2.0	2.0		
Extension of Effective Green, e		2.0	2.0	2.0	2.0		2.0	2.0	2.0	2.0	2.0		
Arrival Type, AT		3	3	3	3		3	3	3	3	3		
Unit Extension, UE		3.0	3.0	3.0	3.0		3.0	3.0	3.0	3.0	3.0		
Filtering/Metering, I		1.000	1.000	1.000	1.000		1.000	1.000	1.000	1.000	1.000		
Initial Unmet Demand, Q _b		0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0	
Lane Width		11.0	12.0	14.0	12.0		11.0	11.0	15.0	13.0	13.0		
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N	
Parking Maneuvers, N _m													
Buses Stopping, N _b		0	0	0	0		0	0	0	0	0		
Min. Time for Pedestrians, G _p		12.4			16.2			15.4			14.9		
Phasing	EW Perm	02		03		04		NS Perm	06		07		08
Timing	G = 16.0	G =		G =		G =		G = 29.0	G =		G =		G =
	Y = 5	Y =		Y =		Y =		Y = 5	Y =		Y =		Y =
Duration of Analysis, T = 0.25							Cycle Length, C = 55.0						

Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		175	114	300	170		100	277	311	121	265		
Lane Group Capacity, c		433	461	381	528		571	959	927	592	967		
v/c Ratio, X		0.40	0.25	0.79	0.32		0.18	0.29	0.34	0.20	0.27		
Total Green Ratio, g/C		0.29	0.29	0.29	0.29		0.53	0.53	0.53	0.53	0.53		
Uniform Delay, d ₁		15.7	14.9	17.9	15.3		6.8	7.2	7.5	6.9	7.2		
Progression Factor, PF		1.000	1.000	1.000	1.000		1.000	1.000	1.000	1.000	1.000		
Delay Calibration, k		0.11	0.11	0.33	0.11		0.11	0.11	0.11	0.11	0.11		
Incremental Delay, d ₂		0.6	0.3	10.5	0.4		0.1	0.2	0.2	0.2	0.2		
Initial Queue Delay, d ₃		0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0		
Control Delay		16.3	15.2	28.5	15.6		6.9	7.4	7.7	7.1	7.3		
Lane Group LOS		B	B	C	B		A	A	A	A	A		
Approach Delay		15.9			23.8			7.5			7.3		
Approach LOS		B			C			A			A		
Intersection Delay		12.9			X _C = 0.50			Intersection LOS			B		

HCS+™ DETAILED REPORT

General Information	Site Information
Analyst <i>JAG</i>	Intersection <i>Route 94/Rt 17 SB ramp</i>
Agency or Co. <i>TMA</i>	Area Type <i>All other areas</i>
Date Performed <i>2/20/09</i>	Jurisdiction <i>Village of Chester</i>
Time Period <i>PM Peak Hour</i>	Analysis Year <i>No Build Condition</i>
	Project ID

Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N _l		1	1	1	1					1	1	0	
Lane Group		T	R	L	T					L	TR		
Volume, V (vph)		293	197	124	554					325	1	94	
% Heavy Vehicles, %HV		7	7	4	4					4	4	4	
Peak-Hour Factor, PHF		0.83	0.83	0.90	0.90					0.79	0.79	0.79	
Pretimed (P) or Actuated (A)		A	A	A	A					A	A	A	
Start-up Lost Time, l ₁		2.0	2.0	2.0	2.0					2.0	2.0		
Extension of Effective Green, e		2.0	2.0	2.0	2.0					2.0	2.0		
Arrival Type, AT		3	3	5	5					3	3		
Unit Extension, UE		3.0	3.0	3.0	3.0					3.0	3.0		
Filtering/Metering, I		1.000	1.000	1.000	1.000					1.000	1.000		
Initial Unmet Demand, Q _b		0.0	0.0	0.0	0.0					0.0	0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0	
Lane Width		12.0	13.0	12.0	12.0					11.0	12.0		
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	5	N	
Parking Maneuvers, N _m													
Buses Stopping, N _b		0	0	0	0					0	0		
Min. Time for Pedestrians, G _p		3.2			3.2			3.2			3.2		
Phasing	EW Perm	WB Only	03		04		SB Only	06		07		08	
Timing	G = 40.0	G = 5.0	G =		G =		G = 40.0	G =		G =		G =	
	Y = 5	Y = 5	Y =		Y =		Y = 5	Y =		Y =		Y =	
Duration of Analysis, T = 0.25							Cycle Length, C = 100.0						

Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		353	237	138	616					411	120		
Lane Group Capacity, c		710	624	479	914					654	606		
v/c Ratio, X		0.50	0.38	0.29	0.67					0.63	0.20		
Total Green Ratio, g/C		0.40	0.40	0.50	0.50					0.40	0.40		
Uniform Delay, d ₁		22.5	21.2	20.6	18.9					24.0	19.5		
Progression Factor, PF		1.000	1.000	0.926	0.333					1.000	1.000		
Delay Calibration, k		0.11	0.11	0.11	0.25					0.21	0.11		
Incremental Delay, d ₂		0.6	0.4	0.3	2.0					1.9	0.2		
Initial Queue Delay, d ₃		0.0	0.0	0.0	0.0					0.0	0.0		
Control Delay		23.0	21.6	19.4	8.3					26.0	19.7		
Lane Group LOS		C	C	B	A					C	B		
Approach Delay		22.5			10.3						24.6		
Approach LOS		C			B						C		
Intersection Delay		18.2			X _C = 0.65			Intersection LOS			B		

HCS+™ DETAILED REPORT

General Information				Site Information			
Analyst	JAG			Intersection	Rt 94/Rt 17 NB ramps		
Agency or Co.	TMA			Area Type	All other areas		
Date Performed	2/20/09			Jurisdiction	Village of Chester		
Time Period	PM Peak Hour			Analysis Year	No Build Condition		
				Project ID			

Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _i	1	1			1	1	0	1	1			
Lane Group	L	T			T	R		LT	R			
Volume, V (vph)	220	378			348	353	343	1	210			
% Heavy Vehicles, %HV	4	4			2	2	5	5	5			
Peak-Hour Factor, PHF	0.88	0.88			0.96	0.96	0.83	0.83	0.83			
Pretimed (P) or Actuated (A)	A	A			A	A	A	A	A			
Start-up Lost Time, l _i	2.0	2.0			2.0	2.0		2.0	2.0			
Extension of Effective Green, e	2.0	2.0			2.0	2.0		2.0	2.0			
Arrival Type, AT	5	5			3	3		3	3			
Unit Extension, UE	3.0	3.0			3.0	3.0		3.0	3.0			
Filtering/Metering, I	1.000	1.000			1.000	1.000		1.000	1.000			
Initial Unmet Demand, Q _b	0.0	0.0			0.0	0.0		0.0	0.0			
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0	0	0	0	
Lane Width	12.0	12.0			11.0	13.0		12.0	13.0			
Parking / Grade / Parking	N	0	N	N	6	N	N	5	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0			0	0		0	0			
Min. Time for Pedestrians, G _p	3.2			3.2			3.2			3.2		
Phasing	EW Perm	EB Only	03	04	NB Only	06	07	08				
Timing	G = 50.0	G = 5.0	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 100.0					

Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	250	430			363	368		414	253			
Lane Group Capacity, c	596	1096			874	794		504	465			
v/c Ratio, X	0.42	0.39			0.42	0.46		0.82	0.54			
Total Green Ratio, g/C	0.60	0.60			0.50	0.50		0.30	0.30			
Uniform Delay, d ₁	15.2	10.5			15.8	16.3		32.5	29.3			
Progression Factor, PF	0.935	0.125			1.000	1.000		1.000	1.000			
Delay Calibration, k	0.11	0.11			0.11	0.11		0.36	0.14			
Incremental Delay, d ₂	0.5	0.2			0.3	0.4		10.5	1.3			
Initial Queue Delay, d ₃	0.0	0.0			0.0	0.0		0.0	0.0			
Control Delay	14.6	1.5			16.1	16.7		43.0	30.6			
Lane Group LOS	B	A			B	B		D	C			
Approach Delay	6.4			16.4			38.3					
Approach LOS	A			B			D					
Intersection Delay	20.1			X _C = 0.60			Intersection LOS			C		

HCS+™ DETAILED REPORT

General Information	Site Information
Analyst <i>JAG</i>	Intersection <i>NYS Rt. 94/ NYS Rt. 17M</i>
Agency or Co. <i>TMA</i>	Area Type <i>All other areas</i>
Date Performed <i>2/20/09</i>	Jurisdiction <i>Village of Chester</i>
Time Period <i>PM Peak Hour</i>	Analysis Year <i>No Build Condition</i>
	Project ID

Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _l	1	1	1	1	1	0	2	1	0	1	2	0
Lane Group	L	T	R	L	TR		L	TR		L	TR	
Volume, V (vph)	227	225	189	50	207	94	312	350	13	95	417	298
% Heavy Vehicles, %HV	3	3	3	5	5	5	6	6	6	3	3	3
Peak-Hour Factor, PHF	0.95	0.95	0.95	0.92	0.92	0.92	0.94	0.94	0.94	0.91	0.91	0.91
Pretimed (P) or Actuated (A)	A	A	A	A	A	A	A	A	A	A	A	A
Start-up Lost Time, l ₁	2.0	2.0	2.0	2.0	2.0		2.0	2.0		2.0	2.0	
Extension of Effective Green, e	2.0	2.0	2.0	2.0	2.0		2.0	2.0		2.0	2.0	
Arrival Type, AT	3	3	3	3	3		3	3		3	3	
Unit Extension, UE	3.0	3.0	3.0	3.0	3.0		3.0	3.0		3.0	3.0	
Filtering/Metering, I	1.000	1.000	1.000	1.000	1.000		1.000	1.000		1.000	1.000	
Initial Unmet Demand, Q _b	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	105
Lane Width	12.0	12.0	12.0	12.0	12.0		10.0	10.0		10.0	12.0	
Parking / Grade / Parking	N	-5	N	N	-3	N	N	-5	N	N	5	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0	0	0	0		0	0		0	0	
Min. Time for Pedestrians, G _p	3.2			3.2			3.2			3.2		
Phasing	EW Perm	EB Only	03	04	Thru & RT	NB Only	SB Only	08				
Timing	G = 27.0	G = 12.0	G =	G =	G = 16.0	G = 14.0	G = 11.0	G =				
	Y = 4	Y = 4	Y =	Y =	Y = 4	Y = 4	Y = 4	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 100.0					

Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	239	237	199	54	327		332	386		104	670	
Lane Group Capacity, c	444	813	691	251	473		443	580		175	881	
v/c Ratio, X	0.54	0.29	0.29	0.22	0.69		0.75	0.67		0.59	0.76	
Total Green Ratio, g/C	0.43	0.43	0.43	0.27	0.27		0.14	0.34		0.11	0.27	
Uniform Delay, d ₁	30.7	18.6	18.5	28.3	32.8		41.3	28.1		42.4	33.5	
Progression Factor, PF	1.000	1.000	1.000	1.000	1.000		1.000	1.000		1.000	1.000	
Delay Calibration, k	0.14	0.11	0.11	0.11	0.26		0.30	0.24		0.18	0.31	
Incremental Delay, d ₂	1.3	0.2	0.2	0.4	4.3		7.0	2.9		5.4	3.9	
Initial Queue Delay, d ₃	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Control Delay	32.0	18.8	18.8	28.7	37.0		48.3	31.0		47.7	37.5	
Lane Group LOS	C	B	B	C	D		D	C		D	D	
Approach Delay	23.4			35.9			39.0			38.8		
Approach LOS	C			D			D			D		
Intersection Delay	34.4			X _C = 0.71			Intersection LOS			C		

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	JAG			Intersection	Hambletonian and High		
Agency/Co.	TMA			Jurisdiction	Chester		
Date Performed	2/17/2009			Analysis Year	No Build Condition		
Analysis Time Period	PM Peak Hour						
Project Description							
East/West Street: Hambletonian Avenue				North/South Street: High Street			
Intersection Orientation: North-South				Study Period (hrs): 0.25			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)	16	303			202	16	
Peak-Hour Factor, PHF	0.93	0.93	1.00	1.00	0.90	0.90	
Hourly Flow Rate, HFR (veh/h)	17	325	0	0	224	17	
Percent Heavy Vehicles	1	--	--	0	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	1	0	
Configuration	LT						TR
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)	38		16				
Peak-Hour Factor, PHF	0.64	1.00	0.64	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	59	0	25	0	0	0	
Percent Heavy Vehicles	4	0	4	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	0	0	0	
Configuration		LR					
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration	LT						LR
v (veh/h)	17						84
C (m) (veh/h)	1331						527
v/c	0.01						0.16
95% queue length	0.04						0.56
Control Delay (s/veh)	7.7						13.1
LOS	A						B
Approach Delay (s/veh)	--	--					13.1
Approach LOS	--	--					B

TWO-WAY STOP CONTROL SUMMARY								
General Information				Site Information				
Analyst	JAG			Intersection	Ward and Rt 17M			
Agency/Co.	TMA			Jurisdiction	Goshen			
Date Performed	2/17/2009			Analysis Year	No Build Condition			
Analysis Time Period	PM Peak Hour							
Project Description								
East/West Street: Ward Road				North/South Street: NYS Route 17M				
Intersection Orientation: North-South				Study Period (hrs): 0.25				
Vehicle Volumes and Adjustments								
Major Street		Northbound			Southbound			
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)		436	15	57	303			
Peak-Hour Factor, PHF	1.00	0.93	0.93	0.84	0.84	1.00		
Hourly Flow Rate, HFR (veh/h)	0	468	16	67	360	0		
Percent Heavy Vehicles	0	--	--	0	--	--		
Median Type	Undivided							
RT Channelized			0			0		
Lanes	0	1	0	0	1	0		
Configuration			TR	LT				
Upstream Signal		0			0			
Minor Street		Eastbound			Westbound			
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)				5		50		
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.81	1.00	0.81		
Hourly Flow Rate, HFR (veh/h)	0	0	0	6	0	61		
Percent Heavy Vehicles	0	0	0	0	0	0		
Percent Grade (%)		0			0			
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	0		
Configuration					LR			
Delay, Queue Length, and Level of Service								
Approach	Northbound	Southbound	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration		LT		LR				
v (veh/h)		67		67				
C (m) (veh/h)		1089		534				
v/c		0.06		0.13				
95% queue length		0.20		0.43				
Control Delay (s/veh)		8.5		12.7				
LOS		A		B				
Approach Delay (s/veh)	--	--	12.7					
Approach LOS	--	--	B					

TWO-WAY STOP CONTROL SUMMARY

General Information			Site Information					
Analyst	JAG		Intersection	Rt 17M and Main				
Agency/Co.	TMA		Jurisdiction	Chester				
Date Performed	2/17/2009		Analysis Year	No Build Condition				
Analysis Time Period	PM Peak Hour							
Project Description								
East/West Street: NYS Route 17M			North/South Street: Main Street					
Intersection Orientation: East-West			Study Period (hrs): 0.25					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	20	582			583	134		
Peak-Hour Factor, PHF	0.84	0.84	1.00	1.00	0.95	0.95		
Hourly Flow Rate, HFR (veh/h)	23	692	0	0	613	141		
Percent Heavy Vehicles	3	--	--	0	--	--		
Median Type	Two Way Left Turn Lane							
RT Channelized			0			0		
Lanes	1	1	0	0	1	0		
Configuration	L	T				TR		
Upstream Signal		0			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)				115		16		
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.60	1.00	0.60		
Hourly Flow Rate, HFR (veh/h)	0	0	0	191	0	26		
Percent Heavy Vehicles	0	0	0	3	0	3		
Percent Grade (%)	0			0				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	0		
Configuration					LR			
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	L						LR	
v (veh/h)	23						217	
C (m) (veh/h)	852						357	
v/c	0.03						0.61	
95% queue length	0.08						3.82	
Control Delay (s/veh)	9.3						29.5	
LOS	A						D	
Approach Delay (s/veh)	--	--					29.5	
Approach LOS	--	--					D	

HCS+™ DETAILED REPORT

General Information		Site Information	
Analyst	JAG	Intersection	Arcadia and Rt 17M
Agency or Co.	TMA	Area Type	All other areas
Date Performed	2/17/2009	Jurisdiction	Goshen
Time Period	PM Peak Hour	Analysis Year	No Build Condition
		Project ID	

Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _l	0		0				0	1			1	0
Lane Group		LR						LT			TR	
Volume, V (vph)	51		72				74	410			317	61
% Heavy Vehicles, %HV	0		0				1	1			0	0
Peak-Hour Factor, PHF	0.91		0.91				0.88	0.88			0.79	0.79
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, l ₁		2.0						2.0			2.0	
Extension of Effective Green, e		2.0						2.0			2.0	
Arrival Type, AT		3						3			3	
Unit Extension, UE		3.0						3.0			3.0	
Filtering/Metering, I		1.000						1.000			1.000	
Initial Unmet Demand, Q _b		0.0						0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0
Lane Width		16.0						12.0			12.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b		0						0			0	
Min. Time for Pedestrians, G _p		3.2			3.2			3.2			3.2	
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 20.0	G =	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					

Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		135						550			478	
Lane Group Capacity, c		648						787			930	
v/c Ratio, X		0.21						0.70			0.51	
Total Green Ratio, g/C		0.33						0.50			0.50	
Uniform Delay, d ₁		14.3						11.5			10.1	
Progression Factor, PF		1.000						1.000			1.000	
Delay Calibration, k		0.50						0.50			0.50	
Incremental Delay, d ₂		0.7						5.1			2.0	
Initial Queue Delay, d ₃		0.0						0.0			0.0	
Control Delay		15.1						16.6			12.1	
Lane Group LOS		B						B			B	
Approach Delay		15.1						16.6			12.1	
Approach LOS		B						B			B	
Intersection Delay		14.6			X _c = 0.50			Intersection LOS			B	

HCS+™ DETAILED REPORT

General Information	Site Information
Analyst <i>JAG</i>	Intersection <i>West Ave./ Route 17M</i>
Agency or Co. <i>TMA</i>	Area Type <i>All other areas</i>
Date Performed <i>2/9/2009</i>	Jurisdiction <i>Village of Chester</i>
Time Period <i>PM Peak Hour</i>	Analysis Year <i>No Build Condition</i>
	Project ID

Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N _i	0	1	1	1	1	0	1	1	1	1	1	0	
Lane Group		LT	R	L	TR		L	T	R	L	TR		
Volume, V (vph)	107	76	132	247	108	47	119	251	249	83	243	45	
% Heavy Vehicles, %HV	3	3	3	1	1	1	0	0	0	0	0	0	
Peak-Hour Factor, PHF	0.81	0.81	0.81	0.88	0.88	0.88	0.94	0.94	0.94	0.98	0.98	0.98	
Pretimed (P) or Actuated (A)	A	A	A	A	A	A	A	A	A	A	A	A	
Start-up Lost Time, l _i		2.0	2.0	2.0	2.0		2.0	2.0	2.0	2.0	2.0		
Extension of Effective Green, e		2.0	2.0	2.0	2.0		2.0	2.0	2.0	2.0	2.0		
Arrival Type, AT		3	3	3	3		3	3	3	3	3		
Unit Extension, UE		3.0	3.0	3.0	3.0		3.0	3.0	3.0	3.0	3.0		
Filtering/Metering, I		1.000	1.000	1.000	1.000		1.000	1.000	1.000	1.000	1.000		
Initial Unmet Demand, Q _b		0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0	
Lane Width		11.0	12.0	14.0	12.0		11.0	11.0	15.0	13.0	13.0		
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N	
Parking Maneuvers, N _m													
Buses Stopping, N _b		0	0	0	0		0	0	0	0	0		
Min. Time for Pedestrians, G _p		12.4			16.2			15.4			14.9		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08					
Timing	G = 16.0	G =	G =	G =	G = 29.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 55.0						

Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		226	163	281	176		127	267	265	85	294		
Lane Group Capacity, c		375	456	347	522		560	969	937	616	1011		
v/c Ratio, X		0.60	0.36	0.81	0.34		0.23	0.28	0.28	0.14	0.29		
Total Green Ratio, g/C		0.29	0.29	0.29	0.29		0.53	0.53	0.53	0.53	0.53		
Uniform Delay, d ₁		16.8	15.4	18.1	15.3		7.0	7.2	7.2	6.6	7.3		
Progression Factor, PF		1.000	1.000	1.000	1.000		1.000	1.000	1.000	1.000	1.000		
Delay Calibration, k		0.19	0.11	0.35	0.11		0.11	0.11	0.11	0.11	0.11		
Incremental Delay, d ₂		2.7	0.5	13.4	0.4		0.2	0.2	0.2	0.1	0.2		
Initial Queue Delay, d ₃		0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0		
Control Delay		19.5	15.9	31.5	15.7		7.2	7.3	7.4	6.7	7.4		
Lane Group LOS		B	B	C	B		A	A	A	A	A		
Approach Delay		18.0			25.4			7.3			7.3		
Approach LOS		B			C			A			A		
Intersection Delay		13.9			X _C = 0.48			Intersection LOS			B		

HCS+™ DETAILED REPORT

General Information				Site Information			
Analyst	JAG			Intersection	Route 94/Rt 17 SB ramp		
Agency or Co.	TMA			Area Type	All other areas		
Date Performed	2/20/09			Jurisdiction	Village of Chester		
Time Period	Saturday Peak Hour			Analysis Year	No Build Condition		
				Project ID			

Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N _l		1	1	1	1					1	1	0	
Lane Group		T	R	L	T					L	TR		
Volume, V (vph)		235	157	159	345					282	1	106	
% Heavy Vehicles, %HV		2	2	1	1					2	2	2	
Peak-Hour Factor, PHF		0.86	0.86	0.91	0.91					0.89	0.89	0.89	
Pretimed (P) or Actuated (A)		A	A	A	A					A	A	A	
Start-up Lost Time, l ₁		2.0	2.0	2.0	2.0					2.0	2.0		
Extension of Effective Green, e		2.0	2.0	2.0	2.0					2.0	2.0		
Arrival Type, AT		3	3	5	5					3	3		
Unit Extension, UE		3.0	3.0	3.0	3.0					3.0	3.0		
Filtering/Metering, I		1.000	1.000	1.000	1.000					1.000	1.000		
Initial Unmet Demand, Q _b		0.0	0.0	0.0	0.0					0.0	0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0	
Lane Width		12.0	13.0	12.0	12.0					11.0	12.0		
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	5	N	
Parking Maneuvers, N _m													
Buses Stopping, N _b		0	0	0	0					0	0		
Min. Time for Pedestrians, G _p		3.2			3.2			3.2			3.2		
Phasing	EW Perm	WB Only	03		04		SB Only	06		07		08	
Timing	G = 40.0	G = 5.0	G =		G =		G = 40.0	G =		G =		G =	
	Y = 5	Y = 5	Y =		Y =		Y = 5	Y =		Y =		Y =	
Duration of Analysis, T = 0.25							Cycle Length, C = 100.0						

Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		273	183	175	379					317	120	
Lane Group Capacity, c		745	654	560	941					667	618	
v/c Ratio, X		0.37	0.28	0.31	0.40					0.48	0.19	
Total Green Ratio, g/C		0.40	0.40	0.50	0.50					0.40	0.40	
Uniform Delay, d ₁		21.1	20.3	18.4	15.7					22.2	19.5	
Progression Factor, PF		1.000	1.000	0.926	0.333					1.000	1.000	
Delay Calibration, k		0.11	0.11	0.11	0.11					0.11	0.11	
Incremental Delay, d ₂		0.3	0.2	0.3	0.3					0.5	0.2	
Initial Queue Delay, d ₃		0.0	0.0	0.0	0.0					0.0	0.0	
Control Delay		21.4	20.5	17.4	5.5					22.8	19.7	
Lane Group LOS		C	C	B	A					C	B	
Approach Delay	21.0			9.3						21.9		
Approach LOS	C			A						C		
Intersection Delay	16.8			X _C = 0.44			Intersection LOS			B		

HCS+™ DETAILED REPORT

General Information	Site Information
Analyst <i>JAG</i>	Intersection <i>Rt 94/Rt 17 NB ramps</i>
Agency or Co. <i>TMA</i>	Area Type <i>All other areas</i>
Date Performed <i>2/20/09</i>	Jurisdiction <i>Village of Chester</i>
Time Period <i>Saturday Peak Hour</i>	Analysis Year <i>No Build Condition</i>
	Project ID

Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _l	1	1			1	1	0	1	1			
Lane Group	L	T			T	R		LT	R			
Volume, V (vph)	84	428			329	343	182	0	170			
% Heavy Vehicles, %HV	1	1			1	1	2	2	2			
Peak-Hour Factor, PHF	0.97	0.97			0.85	0.85	0.94	0.94	0.94			
Pretimed (P) or Actuated (A)	A	A			A	A	A	A	A			
Start-up Lost Time, l ₁	2.0	2.0			2.0	2.0		2.0	2.0			
Extension of Effective Green, e	2.0	2.0			2.0	2.0		2.0	2.0			
Arrival Type, AT	5	5			3	3		3	3			
Unit Extension, UE	3.0	3.0			3.0	3.0		3.0	3.0			
Filtering/Metering, I	1.000	1.000			1.000	1.000		1.000	1.000			
Initial Unmet Demand, Q _b	0.0	0.0			0.0	0.0		0.0	0.0			
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0	0	0	0	
Lane Width	12.0	12.0			11.0	13.0		12.0	13.0			
Parking / Grade / Parking	N	0	N	N	6	N	N	5	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0			0	0		0	0			
Min. Time for Pedestrians, G _p	3.2			3.2			3.2			3.2		
Phasing	EW Perm	EB Only	03	04	NB Only	06	07	08				
Timing	G = 50.0	G = 5.0	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 100.0					

Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	87	441			387	404		194	181			
Lane Group Capacity, c	593	1129			882	802		519	479			
v/c Ratio, X	0.15	0.39			0.44	0.50		0.37	0.38			
Total Green Ratio, g/C	0.60	0.60			0.50	0.50		0.30	0.30			
Uniform Delay, d ₁	13.0	10.4			16.0	16.7		27.6	27.6			
Progression Factor, PF	0.935	0.125			1.000	1.000		1.000	1.000			
Delay Calibration, k	0.11	0.11			0.11	0.11		0.11	0.11			
Incremental Delay, d ₂	0.1	0.2			0.4	0.5		0.5	0.5			
Initial Queue Delay, d ₃	0.0	0.0			0.0	0.0		0.0	0.0			
Control Delay	12.3	1.5			16.4	17.2		28.0	28.1			
Lane Group LOS	B	A			B	B		C	C			
Approach Delay	3.3			16.8			28.1					
Approach LOS	A			B			C					
Intersection Delay	15.1			X _C = 0.41			Intersection LOS			B		

HCS+™ DETAILED REPORT

General Information	Site Information
Analyst <i>JAG</i>	Intersection <i>NYS Rt. 94/ NYS Rt. 17M</i>
Agency or Co. <i>TMA</i>	Area Type <i>All other areas</i>
Date Performed <i>2/20/09</i>	Jurisdiction <i>Village of Chester</i>
Time Period <i>Saturday Peak Hour</i>	Analysis Year <i>No Build Condition</i>
	Project ID

Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _l	1	1	1	1	1	0	2	1	0	1	2	0
Lane Group	L	T	R	L	TR		L	TR		L	TR	
Volume, V (vph)	276	182	291	65	193	122	335	330	28	128	275	211
% Heavy Vehicles, %HV	3	3	3	3	3	3	1	1	1	2	2	2
Peak-Hour Factor, PHF	0.87	0.87	0.87	0.86	0.86	0.86	0.94	0.94	0.94	0.94	0.94	0.94
Pretimed (P) or Actuated (A)	A	A	A	A	A	A	A	A	A	A	A	A
Start-up Lost Time, l ₁	2.0	2.0	2.0	2.0	2.0		2.0	2.0		2.0	2.0	
Extension of Effective Green, e	2.0	2.0	2.0	2.0	2.0		2.0	2.0		2.0	2.0	
Arrival Type, AT	3	3	3	3	3		3	3		3	3	
Unit Extension, UE	3.0	3.0	3.0	3.0	3.0		3.0	3.0		3.0	3.0	
Filtering/Metering, I	1.000	1.000	1.000	1.000	1.000		1.000	1.000		1.000	1.000	
Initial Unmet Demand, Q _b	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	70
Lane Width	12.0	12.0	12.0	12.0	12.0		10.0	10.0		10.0	12.0	
Parking / Grade / Parking	N	-5	N	N	-3	N	N	-5	N	N	5	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0	0	0	0		0	0		0	0	
Min. Time for Pedestrians, G _p	3.2			3.2			3.2			3.2		
Phasing	EW Perm	EB Only	03	04	NS Perm	NB Only	SB Only	08				
Timing	G = 25.0	G = 19.0	G =	G =	G = 12.0	G = 14.0	G = 10.0	G =				
	Y = 4	Y = 4	Y =	Y =	Y = 4	Y = 4	Y = 4	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 100.0					

Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	317	209	334	76	366		356	381		136	443	
Lane Group Capacity, c	514	908	771	260	441		465	533		233	722	
v/c Ratio, X	0.62	0.23	0.43	0.29	0.83		0.77	0.71		0.58	0.61	
Total Green Ratio, g/C	0.48	0.48	0.48	0.25	0.25		0.14	0.30		0.22	0.22	
Uniform Delay, d ₁	30.2	15.2	17.1	30.3	35.5		41.4	31.2		40.2	35.2	
Progression Factor, PF	1.000	1.000	1.000	1.000	1.000		1.000	1.000		1.000	1.000	
Delay Calibration, k	0.20	0.11	0.11	0.11	0.37		0.32	0.28		0.18	0.20	
Incremental Delay, d ₂	2.2	0.1	0.4	0.6	12.6		7.5	4.5		3.7	1.6	
Initial Queue Delay, d ₃	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Control Delay	32.5	15.3	17.5	31.0	48.1		48.9	35.7		44.0	36.7	
Lane Group LOS	C	B	B	C	D		D	D		D	D	
Approach Delay	22.5			45.1			42.1			38.4		
Approach LOS	C			D			D			D		
Intersection Delay	35.4			X _C = 0.73			Intersection LOS			D		

TWO-WAY STOP CONTROL SUMMARY								
General Information				Site Information				
Analyst	JAG			Intersection	Hambletonian and High			
Agency/Co.	TMA			Jurisdiction	Chester			
Date Performed	2/17/2009			Analysis Year	No Build Condition			
Analysis Time Period	Saturday Peak Hour							
Project Description								
East/West Street: Hambletonian Avenue				North/South Street: High Street				
Intersection Orientation: North-South				Study Period (hrs): 0.25				
Vehicle Volumes and Adjustments								
Major Street	Northbound			Southbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	19	276			299	30		
Peak-Hour Factor, PHF	0.91	0.91	1.00	1.00	0.88	0.88		
Hourly Flow Rate, HFR (veh/h)	20	303	0	0	339	34		
Percent Heavy Vehicles	1	--	--	0	--	--		
Median Type	Undivided							
RT Channelized			0			0		
Lanes	0	1	0	0	1	0		
Configuration	LT					TR		
Upstream Signal		0			0			
Minor Street	Eastbound			Westbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)	42		24					
Peak-Hour Factor, PHF	0.70	1.00	0.70	1.00	1.00	1.00		
Hourly Flow Rate, HFR (veh/h)	60	0	34	0	0	0		
Percent Heavy Vehicles	4	0	4	0	0	0		
Percent Grade (%)		0			0			
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	0		
Configuration		LR						
Delay, Queue Length, and Level of Service								
Approach	Northbound	Southbound	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	LT						LR	
v (veh/h)	20						94	
C (m) (veh/h)	1191						467	
v/c	0.02						0.20	
95% queue length	0.05						0.74	
Control Delay (s/veh)	8.1						14.6	
LOS	A						B	
Approach Delay (s/veh)	--	--					14.6	
Approach LOS	--	--					B	

TWO-WAY STOP CONTROL SUMMARY								
General Information				Site Information				
Analyst	JAG			Intersection	Ward and Rt 17M			
Agency/Co.	TMA			Jurisdiction	Goshen			
Date Performed	2/17/2009			Analysis Year	No Build Condition			
Analysis Time Period	Saturday Peak Hour							
Project Description								
East/West Street: Ward Road				North/South Street: NYS Route 17M				
Intersection Orientation: North-South				Study Period (hrs): 0.25				
Vehicle Volumes and Adjustments								
Major Street		Northbound			Southbound			
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)		356	7	30	278			
Peak-Hour Factor, PHF	1.00	0.91	0.91	0.86	0.86	1.00		
Hourly Flow Rate, HFR (veh/h)	0	391	7	34	323	0		
Percent Heavy Vehicles	0	--	--	0	--	--		
Median Type	Undivided							
RT Channelized			0					0
Lanes	0	1	0	0	1	0		
Configuration			TR	LT				
Upstream Signal		0			0			
Minor Street		Eastbound			Westbound			
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)				2		14		
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.63	1.00	0.63		
Hourly Flow Rate, HFR (veh/h)	0	0	0	3	0	22		
Percent Heavy Vehicles	0	0	0	0	0	0		
Percent Grade (%)		0			0			
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	0		
Configuration					LR			
Delay, Queue Length, and Level of Service								
Approach	Northbound	Southbound	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration		LT		LR				
v (veh/h)		34		25				
C (m) (veh/h)		1172		597				
v/c		0.03		0.04				
95% queue length		0.09		0.13				
Control Delay (s/veh)		8.2		11.3				
LOS		A		B				
Approach Delay (s/veh)	--	--	11.3					
Approach LOS	--	--	B					

TWO-WAY STOP CONTROL SUMMARY								
General Information				Site Information				
Analyst	JAG			Intersection	Rt 17M and Main			
Agency/Co.	TMA			Jurisdiction	Chester			
Date Performed	2/17/2009			Analysis Year	No Build Condition			
Analysis Time Period	Saturday Peak Hour							
Project Description								
East/West Street: NYS Route 17M				North/South Street: Main Street				
Intersection Orientation: East-West				Study Period (hrs): 0.25				
Vehicle Volumes and Adjustments								
Major Street		Eastbound			Westbound			
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	17	426			573	99		
Peak-Hour Factor, PHF	0.95	0.95	1.00	1.00	0.98	0.98		
Hourly Flow Rate, HFR (veh/h)	17	448	0	0	584	101		
Percent Heavy Vehicles	3	--	--	0	--	--		
Median Type	Two Way Left Turn Lane							
RT Channelized			0					0
Lanes	1	1	0	0	1	0		
Configuration	L	T						TR
Upstream Signal		0			0			
Minor Street		Northbound			Southbound			
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)				65		36		
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.98	1.00	0.98		
Hourly Flow Rate, HFR (veh/h)	0	0	0	66	0	36		
Percent Heavy Vehicles	0	0	0	2	0	2		
Percent Grade (%)		0			0			
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	0		
Configuration					LR			
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	L						LR	
v (veh/h)	17						102	
C (m) (veh/h)	904						445	
v/c	0.02						0.23	
95% queue length	0.06						0.87	
Control Delay (s/veh)	9.1						15.5	
LOS	A						C	
Approach Delay (s/veh)	--	--					15.5	
Approach LOS	--	--					C	

HCS+™ DETAILED REPORT

General Information		Site Information	
Analyst	JAG	Intersection	Arcadia and Rt 17M
Agency or Co.	TMA	Area Type	All other areas
Date Performed	2/17/2009	Jurisdiction	Goshen
Time Period	Saturday Peak Hour	Analysis Year	No Build Condition
		Project ID	

Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _l	0		0				0	1			1	0
Lane Group		LR						LT			TR	
Volume, V (vph)	45		50				57	302			241	54
% Heavy Vehicles, %HV	0		0				0	0			0	0
Peak-Hour Factor, PHF	0.94		0.94				0.96	0.96			0.80	0.80
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, l ₁		2.0						2.0			2.0	
Extension of Effective Green, e		2.0						2.0			2.0	
Arrival Type, AT		3						3			3	
Unit Extension, UE		3.0						3.0			3.0	
Filtering/Metering, I		1.000						1.000			1.000	
Initial Unmet Demand, Q _b		0.0						0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0
Lane Width		16.0						12.0			12.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b		0						0			0	
Min. Time for Pedestrians, G _p		3.2			3.2			3.2			3.2	
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 20.0	G =	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					

Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		101						374			368	
Lane Group Capacity, c		651						853			927	
v/c Ratio, X		0.16						0.44			0.40	
Total Green Ratio, g/C		0.33						0.50			0.50	
Uniform Delay, d ₁		14.1						9.6			9.4	
Progression Factor, PF		1.000						1.000			1.000	
Delay Calibration, k		0.50						0.50			0.50	
Incremental Delay, d ₂		0.5						1.6			1.3	
Initial Queue Delay, d ₃		0.0						0.0			0.0	
Control Delay		14.6						11.2			10.6	
Lane Group LOS		B						B			B	
Approach Delay		14.6						11.2			10.6	
Approach LOS		B						B			B	
Intersection Delay		11.4			X _C = 0.33			Intersection LOS			B	

HCS+™ DETAILED REPORT

General Information	Site Information
Analyst <i>JAG</i>	Intersection <i>West Ave./ Route 17M</i>
Agency or Co. <i>TMA</i>	Area Type <i>All other areas</i>
Date Performed <i>2/9/2009</i>	Jurisdiction <i>Village of Chester</i>
Time Period <i>Saturday Peak Hour</i>	Analysis Year <i>No Build Condition</i>
	Project ID

Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N _l	0	1	1	1	1	0	1	1	1	1	1	0	
Lane Group		LT	R	L	TR		L	T	R	L	TR		
Volume, V (vph)	61	126	103	273	119	34	81	252	380	100	176	64	
% Heavy Vehicles, %HV	3	3	3	1	1	1	2	2	2	2	2	2	
Peak-Hour Factor, PHF	0.81	0.81	0.81	0.95	0.95	0.95	0.95	0.95	0.95	0.92	0.92	0.92	
Pretimed (P) or Actuated (A)	A	A	A	A	A	A	A	A	A	A	A	A	
Start-up Lost Time, l ₁		2.0	2.0	2.0	2.0		2.0	2.0	2.0	2.0	2.0		
Extension of Effective Green, e		2.0	2.0	2.0	2.0		2.0	2.0	2.0	2.0	2.0		
Arrival Type, AT		3	3	3	3		3	3	3	3	3		
Unit Extension, UE		3.0	3.0	3.0	3.0		3.0	3.0	3.0	3.0	3.0		
Filtering/Metering, I		1.000	1.000	1.000	1.000		1.000	1.000	1.000	1.000	1.000		
Initial Unmet Demand, Q _b		0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0	
Lane Width		11.0	12.0	14.0	12.0		11.0	11.0	15.0	13.0	13.0		
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N	
Parking Maneuvers, N _m													
Buses Stopping, N _b		0	0	0	0		0	0	0	0	0		
Min. Time for Pedestrians, G _p		12.4			16.2			15.4			14.9		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08					
Timing	G = 16.0	G =	G =	G =	G = 29.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 55.0						

Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		231	127	287	161		85	265	400	109	261		
Lane Group Capacity, c		436	456	342	529		568	950	919	605	974		
v/c Ratio, X		0.53	0.28	0.84	0.30		0.15	0.28	0.44	0.18	0.27		
Total Green Ratio, g/C		0.29	0.29	0.29	0.29		0.53	0.53	0.53	0.53	0.53		
Uniform Delay, d ₁		16.3	15.0	18.3	15.2		6.7	7.2	8.0	6.8	7.2		
Progression Factor, PF		1.000	1.000	1.000	1.000		1.000	1.000	1.000	1.000	1.000		
Delay Calibration, k		0.13	0.11	0.37	0.11		0.11	0.11	0.11	0.11	0.11		
Incremental Delay, d ₂		1.2	0.3	16.7	0.3		0.1	0.2	0.3	0.1	0.1		
Initial Queue Delay, d ₃		0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0		
Control Delay		17.6	15.4	35.0	15.5		6.8	7.4	8.3	6.9	7.3		
Lane Group LOS		B	B	D	B		A	A	A	A	A		
Approach Delay		16.8			28.0			7.8			7.2		
Approach LOS		B			C			A			A		
Intersection Delay		14.1			X _C = 0.58			Intersection LOS			B		

HCS+™ DETAILED REPORT

General Information	Site Information
Analyst <i>JAG</i>	Intersection <i>Route 94/Rt 17 SB ramp</i>
Agency or Co. <i>TMA</i>	Area Type <i>All other areas</i>
Date Performed <i>2/20/09</i>	Jurisdiction <i>Village of Chester</i>
Time Period <i>AM Peak Hour</i>	Analysis Year <i>Build Condition</i>
	Project ID

Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N _l		1	1	1	1					1	1	0	
Lane Group		T	R	L	T					L	TR		
Volume, V (vph)		171	288	307	216					279	1	406	
% Heavy Vehicles, %HV		8	8	5	5					5	5	5	
Peak-Hour Factor, PHF		0.94	0.94	0.78	0.78					0.71	0.71	0.71	
Pretimed (P) or Actuated (A)		A	A	A	A					A	A	A	
Start-up Lost Time, l ₁		2.0	2.0	2.0	2.0					2.0	2.0		
Extension of Effective Green, e		2.0	2.0	2.0	2.0					2.0	2.0		
Arrival Type, AT		3	3	5	5					3	3		
Unit Extension, UE		3.0	3.0	3.0	3.0					3.0	3.0		
Filtering/Metering, I		1.000	1.000	1.000	1.000					1.000	1.000		
Initial Unmet Demand, Q _b		0.0	0.0	0.0	0.0					0.0	0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0	
Lane Width		12.0	13.0	12.0	12.0					11.0	12.0		
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	5	N	
Parking Maneuvers, N _m													
Buses Stopping, N _b		0	0	0	0					0	0		
Min. Time for Pedestrians, G _p		3.2			3.2			3.2			3.2		
Phasing	EW Perm	WB Only	03		04		SB Only	06		07		08	
Timing	G = 38.0	G = 5.0	G =		G =		G = 42.0	G =		G =		G =	
	Y = 5	Y = 5	Y =		Y =		Y = 5	Y =		Y =		Y =	
Duration of Analysis, T = 0.25							Cycle Length, C = 100.0						

Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		182	306	394	277					393	573	
Lane Group Capacity, c		668	587	591	869					680	630	
v/c Ratio, X		0.27	0.52	0.67	0.32					0.58	0.91	
Total Green Ratio, g/C		0.38	0.38	0.48	0.48					0.42	0.42	
Uniform Delay, d ₁		21.4	24.0	22.0	16.0					22.2	27.2	
Progression Factor, PF		1.000	1.000	0.926	0.385					1.000	1.000	
Delay Calibration, k		0.11	0.13	0.24	0.11					0.17	0.43	
Incremental Delay, d ₂		0.2	0.8	2.9	0.2					1.2	17.3	
Initial Queue Delay, d ₃		0.0	0.0	0.0	0.0					0.0	0.0	
Control Delay		21.7	24.8	23.3	6.4					23.4	44.5	
Lane Group LOS		C	C	C	A					C	D	
Approach Delay	23.6			16.3						35.9		
Approach LOS	C			B						D		
Intersection Delay	26.9			X _C = 0.82			Intersection LOS			C		

HCS+™ DETAILED REPORT

General Information	Site Information
Analyst <i>JAG</i>	Intersection <i>Rt 94/Rt 17 NB ramps</i>
Agency or Co. <i>TMA</i>	Area Type <i>All other areas</i>
Date Performed <i>2/20/09</i>	Jurisdiction <i>Village of Chester</i>
Time Period <i>AM Peak Hour</i>	Analysis Year <i>Build Condition</i>
	Project ID

Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _l	1	1			1	1	0	1	1			
Lane Group	L	T			T	R		LT	R			
Volume, V (vph)	112	381			424	375	163	1	98			
% Heavy Vehicles, %HV	5	5			2	2	8	8	8			
Peak-Hour Factor, PHF	0.91	0.91			0.94	0.94	0.77	0.77	0.77			
Pretimed (P) or Actuated (A)	A	A			A	A	A	A	A			
Start-up Lost Time, l ₁	2.0	2.0			2.0	2.0		2.0	2.0			
Extension of Effective Green, e	2.0	2.0			2.0	2.0		2.0	2.0			
Arrival Type, AT	5	5			3	3		3	3			
Unit Extension, UE	3.0	3.0			3.0	3.0		3.0	3.0			
Filtering/Metering, I	1.000	1.000			1.000	1.000		1.000	1.000			
Initial Unmet Demand, Q _b	0.0	0.0			0.0	0.0		0.0	0.0			
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0	0	0	0	
Lane Width	12.0	12.0			11.0	13.0		12.0	13.0			
Parking / Grade / Parking	N	0	N	N	6	N	N	5	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0			0	0		0	0			
Min. Time for Pedestrians, G _p	3.2			3.2			3.2			3.2		
Phasing	EW Perm	EB Only	03	04	NB Only	06	07	08				
Timing	G = 50.0	G = 5.0	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 100.0					

Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	123	419			451	399		213	127			
Lane Group Capacity, c	520	1086			874	794		490	452			
v/c Ratio, X	0.24	0.39			0.52	0.50		0.43	0.28			
Total Green Ratio, g/C	0.60	0.60			0.50	0.50		0.30	0.30			
Uniform Delay, d ₁	15.5	10.4			16.8	16.7		28.2	26.8			
Progression Factor, PF	0.935	0.125			1.000	1.000		1.000	1.000			
Delay Calibration, k	0.11	0.11			0.12	0.11		0.11	0.11			
Incremental Delay, d ₂	0.2	0.2			0.5	0.5		0.6	0.3			
Initial Queue Delay, d ₃	0.0	0.0			0.0	0.0		0.0	0.0			
Control Delay	14.7	1.5			17.4	17.2		28.8	27.1			
Lane Group LOS	B	A			B	B		C	C			
Approach Delay	4.5			17.3			28.2					
Approach LOS	A			B			C					
Intersection Delay	15.4			X _C = 0.43			Intersection LOS			B		

HCS+™ DETAILED REPORT

General Information	Site Information
Analyst <i>JAG</i>	Intersection <i>NYS Rt. 94/ NYS Rt. 17M</i>
Agency or Co. <i>TMA</i>	Area Type <i>All other areas</i>
Date Performed <i>2/20/09</i>	Jurisdiction <i>Village of Chester</i>
Time Period <i>AM Peak Hour</i>	Analysis Year <i>Build Condition</i>
	Project ID

Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _i	1	1	1	1	1	0	2	1	0	1	2	0
Lane Group	L	T	R	L	TR		L	TR		L	TR	
Volume, V (vph)	105	108	196	40	183	67	307	266	8	45	216	284
% Heavy Vehicles, %HV	13	13	13	6	6	6	6	6	6	7	7	7
Peak-Hour Factor, PHF	0.83	0.83	0.83	0.86	0.86	0.86	0.92	0.92	0.92	0.86	0.86	0.86
Pretimed (P) or Actuated (A)	A	A	A	A	A	A	A	A	A	A	A	A
Start-up Lost Time, l _i	2.0	2.0	2.0	2.0	2.0		2.0	2.0		2.0	2.0	
Extension of Effective Green, e	2.0	2.0	2.0	2.0	2.0		2.0	2.0		2.0	2.0	
Arrival Type, AT	3	3	3	3	3		3	3		3	3	
Unit Extension, UE	3.0	3.0	3.0	3.0	3.0		3.0	3.0		3.0	3.0	
Filtering/Metering, I	1.000	1.000	1.000	1.000	1.000		1.000	1.000		1.000	1.000	
Initial Unmet Demand, Q _b	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width	12.0	12.0	12.0	12.0	12.0		10.0	10.0		10.0	12.0	
Parking / Grade / Parking	N	-5	N	N	-3	N	N	-5	N	N	5	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0	0	0	0		0	0		0	0	
Min. Time for Pedestrians, G _p	3.2			3.2			3.2			3.2		
Phasing	EW Perm	EB Only	03	04	NS Perm	NB Only	SB Only	08				
Timing	G = 27.0	G = 12.0	G =	G =	G = 16.0	G = 14.0	G = 11.0	G =				
	Y = 4	Y = 4	Y =	Y =	Y = 4	Y = 4	Y = 4	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 100.0					

Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	127	130	236	47	291		334	298		52	581	
Lane Group Capacity, c	432	741	630	326	471		443	580		241	814	
v/c Ratio, X	0.29	0.18	0.37	0.14	0.62		0.75	0.51		0.22	0.71	
Total Green Ratio, g/C	0.43	0.43	0.43	0.27	0.27		0.14	0.34		0.27	0.27	
Uniform Delay, d ₁	25.8	17.6	19.4	27.7	32.0		41.3	26.4		35.7	33.0	
Progression Factor, PF	1.000	1.000	1.000	1.000	1.000		1.000	1.000		1.000	1.000	
Delay Calibration, k	0.11	0.11	0.11	0.11	0.20		0.31	0.12		0.11	0.28	
Incremental Delay, d ₂	0.4	0.1	0.4	0.2	2.5		7.2	0.8		0.5	3.0	
Initial Queue Delay, d ₃	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Control Delay	26.1	17.7	19.7	27.9	34.4		48.5	27.2		36.1	36.0	
Lane Group LOS	C	B	B	C	C		D	C		D	D	
Approach Delay	20.8			33.5			38.5			36.0		
Approach LOS	C			C			D			D		
Intersection Delay	32.8			X _C = 0.57			Intersection LOS			C		

TWO-WAY STOP CONTROL SUMMARY								
General Information				Site Information				
Analyst	JAG			Intersection	Hambletonian and High			
Agency/Co.	TMA			Jurisdiction	Chester			
Date Performed	2/17/2009			Analysis Year	Build Condition			
Analysis Time Period	AM Peak Hour							
Project Description								
East/West Street: Hambletonian Avenue				North/South Street: High Street				
Intersection Orientation: North-South				Study Period (hrs): 0.25				
Vehicle Volumes and Adjustments								
Major Street	Northbound			Southbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	112	200			346	123		
Peak-Hour Factor, PHF	0.95	0.95	1.00	1.00	0.89	0.89		
Hourly Flow Rate, HFR (veh/h)	117	210	0	0	388	138		
Percent Heavy Vehicles	10	--	--	0	--	--		
Median Type	Undivided							
RT Channelized			0			0		
Lanes	0	1	0	0	1	0		
Configuration	LT						TR	
Upstream Signal		0			0			
Minor Street	Eastbound			Westbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)	68		106					
Peak-Hour Factor, PHF	0.60	1.00	0.60	1.00	1.00	1.00		
Hourly Flow Rate, HFR (veh/h)	113	0	176	0	0	0		
Percent Heavy Vehicles	12	0	12	0	0	0		
Percent Grade (%)	0			0				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	0		
Configuration		LR						
Delay, Queue Length, and Level of Service								
Approach	Northbound	Southbound	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	LT						LR	
v (veh/h)	117						289	
C (m) (veh/h)	1001						393	
v/c	0.12						0.74	
95% queue length	0.40						5.77	
Control Delay (s/veh)	9.1						35.7	
LOS	A						E	
Approach Delay (s/veh)	--	--					35.7	
Approach LOS	--	--					E	

TWO-WAY STOP CONTROL SUMMARY								
General Information				Site Information				
Analyst	JAG			Intersection	Ward and Rt 17M			
Agency/Co.	TMA			Jurisdiction	Goshen			
Date Performed	2/17/2009			Analysis Year	Build Condition			
Analysis Time Period	AM Peak Hour							
Project Description								
East/West Street: Ward Road				North/South Street: NYS Route 17M				
Intersection Orientation: North-South				Study Period (hrs): 0.25				
Vehicle Volumes and Adjustments								
Major Street	Northbound			Southbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)		263	3	25	254			
Peak-Hour Factor, PHF	1.00	0.87	0.87	0.92	0.92	1.00		
Hourly Flow Rate, HFR (veh/h)	0	302	3	27	276	0		
Percent Heavy Vehicles	0	--	--	1	--	--		
Median Type	Undivided							
RT Channelized			0			0		
Lanes	0	1	0	0	1	0		
Configuration			TR	LT				
Upstream Signal		0			0			
Minor Street	Eastbound			Westbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)				3		69		
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.89	1.00	0.89		
Hourly Flow Rate, HFR (veh/h)	0	0	0	3	0	77		
Percent Heavy Vehicles	0	0	0	1	0	1		
Percent Grade (%)		0			0			
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	0		
Configuration					LR			
Delay, Queue Length, and Level of Service								
Approach	Northbound	Southbound	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration		LT		LR				
v (veh/h)		27		80				
C (m) (veh/h)		1262		719				
v/c		0.02		0.11				
95% queue length		0.07		0.37				
Control Delay (s/veh)		7.9		10.6				
LOS		A		B				
Approach Delay (s/veh)	--	--	10.6					
Approach LOS	--	--	B					

TWO-WAY STOP CONTROL SUMMARY								
General Information				Site Information				
Analyst	JAG			Intersection	Rt 17M and Main			
Agency/Co.	TMA			Jurisdiction	Chester			
Date Performed	2/17/2009			Analysis Year	Build Condition			
Analysis Time Period	AM Peak Hour							
Project Description								
East/West Street: NYS Route 17M				North/South Street: Main Street				
Intersection Orientation: East-West				Study Period (hrs): 0.25				
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	5	338			503	82		
Peak-Hour Factor, PHF	0.94	0.94	1.00	1.00	0.85	0.85		
Hourly Flow Rate, HFR (veh/h)	5	359	0	0	591	96		
Percent Heavy Vehicles	7	--	--	0	--	--		
Median Type	Two Way Left Turn Lane							
RT Channelized			0			0		
Lanes	1	1	0	0	1	0		
Configuration	L	T				TR		
Upstream Signal		0			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)				105		13		
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.91	1.00	0.91		
Hourly Flow Rate, HFR (veh/h)	0	0	0	115	0	14		
Percent Heavy Vehicles	0	0	0	1	0	1		
Percent Grade (%)	0			0				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	0		
Configuration					LR			
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	L						LR	
v (veh/h)	5						129	
C (m) (veh/h)	884						457	
v/c	0.01						0.28	
95% queue length	0.02						1.15	
Control Delay (s/veh)	9.1						15.9	
LOS	A						C	
Approach Delay (s/veh)	--	--					15.9	
Approach LOS	--	--					C	

HCS+™ DETAILED REPORT

General Information	Site Information
Analyst JAG	Intersection Arcadia and Rt 17M
Agency or Co. TMA	Area Type All other areas
Date Performed 2/17/2009	Jurisdiction Goshen
Time Period AM Peak Hour	Analysis Year Build Condition
	Project ID

Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _l	0		0				0	1			1	0
Lane Group		LR						LT			TR	
Volume, V (vph)	56		63				32	294			237	29
% Heavy Vehicles, %HV	1		1				1	1			3	3
Peak-Hour Factor, PHF	0.74		0.74				0.95	0.95			0.92	0.92
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, l ₁		2.0						2.0			2.0	
Extension of Effective Green, e		2.0						2.0			2.0	
Arrival Type, AT		3						3			3	
Unit Extension, UE		3.0						3.0			3.0	
Filtering/Metering, I		1.000						1.000			1.000	
Initial Unmet Demand, Q _b		0.0						0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0
Lane Width		16.0						12.0			12.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b		0						0			0	
Min. Time for Pedestrians, G _p		3.2			3.2			3.2			3.2	
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 20.0	G =	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					

Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		161						343			290	
Lane Group Capacity, c		645						895			909	
v/c Ratio, X		0.25						0.38			0.32	
Total Green Ratio, g/C		0.33						0.50			0.50	
Uniform Delay, d ₁		14.5						9.3			8.9	
Progression Factor, PF		1.000						1.000			1.000	
Delay Calibration, k		0.50						0.50			0.50	
Incremental Delay, d ₂		0.9						1.2			0.9	
Initial Queue Delay, d ₃		0.0						0.0			0.0	
Control Delay		15.5						10.5			9.8	
Lane Group LOS		B						B			A	
Approach Delay		15.5						10.5			9.8	
Approach LOS		B						B			A	
Intersection Delay		11.3			X _C = 0.33			Intersection LOS			B	

HCS+™ DETAILED REPORT

General Information	Site Information
Analyst <i>JAG</i>	Intersection <i>West Ave./ Route 17M</i>
Agency or Co. <i>TMA</i>	Area Type <i>All other areas</i>
Date Performed <i>2/9/2009</i>	Jurisdiction <i>Village of Chester</i>
Time Period <i>AM Peak Hour</i>	Analysis Year <i>Build Condition</i>
	Project ID

Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N _i	0	1	1	1	1	0	1	1	1	1	1	0	
Lane Group		LT	R	L	TR		L	T	R	L	TR		
Volume, V (vph)	66	101	105	228	91	39	81	258	252	113	273	77	
% Heavy Vehicles, %HV	2	2	2	0	0	0	1	1	1	3	3	3	
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.76	0.76	0.76	0.81	0.81	0.81	0.85	0.85	0.85	
Pretimed (P) or Actuated (A)	A	A	A	A	A	A	A	A	A	A	A	A	
Start-up Lost Time, l _i		2.0	2.0	2.0	2.0		2.0	2.0	2.0	2.0	2.0		
Extension of Effective Green, e		2.0	2.0	2.0	2.0		2.0	2.0	2.0	2.0	2.0		
Arrival Type, AT		3	3	3	3		3	3	3	3	3		
Unit Extension, UE		3.0	3.0	3.0	3.0		3.0	3.0	3.0	3.0	3.0		
Filtering/Metering, I		1.000	1.000	1.000	1.000		1.000	1.000	1.000	1.000	1.000		
Initial Unmet Demand, Q _b		0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0	
Lane Width		11.0	12.0	14.0	12.0		11.0	11.0	15.0	13.0	13.0		
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N	
Parking Maneuvers, N _m													
Buses Stopping, N _b		0	0	0	0		0	0	0	0	0		
Min. Time for Pedestrians, G _p		12.4			16.2			15.4			14.9		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08					
Timing	G = 16.0	G =	G =	G =	G = 29.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 55.0						

Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		182	114	300	171		100	319	311	133	412		
Lane Group Capacity, c		425	461	379	528		452	959	927	558	972		
v/c Ratio, X		0.43	0.25	0.79	0.32		0.22	0.33	0.34	0.24	0.42		
Total Green Ratio, g/C		0.29	0.29	0.29	0.29		0.53	0.53	0.53	0.53	0.53		
Uniform Delay, d ₁		15.8	14.9	18.0	15.3		7.0	7.5	7.5	7.0	7.9		
Progression Factor, PF		1.000	1.000	1.000	1.000		1.000	1.000	1.000	1.000	1.000		
Delay Calibration, k		0.11	0.11	0.34	0.11		0.11	0.11	0.11	0.11	0.11		
Incremental Delay, d ₂		0.7	0.3	10.9	0.4		0.2	0.2	0.2	0.2	0.3		
Initial Queue Delay, d ₃		0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0		
Control Delay		16.5	15.2	28.9	15.6		7.2	7.7	7.7	7.3	8.2		
Lane Group LOS		B	B	C	B		A	A	A	A	A		
Approach Delay		16.0			24.1			7.6			8.0		
Approach LOS		B			C			A			A		
Intersection Delay		12.7			X _C = 0.55			Intersection LOS			B		

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	JAG			Intersection	Site and Rte 17M		
Agency/Co.	TMA			Jurisdiction	Chester		
Date Performed	2/25/2009			Analysis Year	Build Condition		
Analysis Time Period	AM Peak Hour						
Project Description							
East/West Street: Site Access				North/South Street: NYS Route 17M			
Intersection Orientation: North-South				Study Period (hrs): 0.25			
Vehicle Volumes and Adjustments							
Major Street		Northbound			Southbound		
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		245	41	10	255		
Peak-Hour Factor, PHF	1.00	0.83	0.83	0.89	0.89	1.00	
Hourly Flow Rate, HFR (veh/h)	0	295	49	11	286	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Undivided						
RT Channelized			0			0	
Lanes	0	1	0	0	1	0	
Configuration			TR	LT			
Upstream Signal		0			0		
Minor Street		Eastbound			Westbound		
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				135		29	
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.90	1.00	0.90	
Hourly Flow Rate, HFR (veh/h)	0	0	0	150	0	32	
Percent Heavy Vehicles	0	0	0	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0			0	
Lanes	0	0	0	0	0	0	
Configuration					LR		
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration		LT		LR			
v (veh/h)		11		182			
C (m) (veh/h)		1226		478			
v/c		0.01		0.38			
95% queue length		0.03		1.76			
Control Delay (s/veh)		8.0		17.1			
LOS		A		C			
Approach Delay (s/veh)	--	--	17.1				
Approach LOS	--	--	C				

HCS+™ DETAILED REPORT

General Information						Site Information							
Analyst JAG						Intersection Route 94/Rt 17 SB ramp							
Agency or Co. TMA						Area Type All other areas							
Date Performed 2/20/09						Jurisdiction Village of Chester							
Time Period PM Peak Hour						Analysis Year Build Condition							
Project ID													
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N _l		1	1	1	1					1	1	0	
Lane Group		T	R	L	T					L	TR		
Volume, V (vph)		293	197	129	554					353	1	94	
% Heavy Vehicles, %HV		7	7	4	4					4	4	4	
Peak-Hour Factor, PHF		0.83	0.83	0.90	0.90					0.79	0.79	0.79	
Pretimed (P) or Actuated (A)		A	A	A	A					A	A	A	
Start-up Lost Time, l ₁		2.0	2.0	2.0	2.0					2.0	2.0		
Extension of Effective Green, e		2.0	2.0	2.0	2.0					2.0	2.0		
Arrival Type, AT		3	3	5	5					3	3		
Unit Extension, UE		3.0	3.0	3.0	3.0					3.0	3.0		
Filtering/Metering, I		1.000	1.000	1.000	1.000					1.000	1.000		
Initial Unmet Demand, Q _b		0.0	0.0	0.0	0.0					0.0	0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0	
Lane Width		12.0	13.0	12.0	12.0					11.0	12.0		
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	5	N	
Parking Maneuvers, N _m													
Buses Stopping, N _b		0	0	0	0					0	0		
Min. Time for Pedestrians, G _p		3.2			3.2			3.2			3.2		
Phasing	EW Perm	WB Only	03		04		SB Only	06		07		08	
Timing	G = 40.0	G = 5.0	G =		G =		G = 40.0	G =		G =		G =	
	Y = 5	Y = 5	Y =		Y =		Y = 5	Y =		Y =		Y =	
Duration of Analysis, T = 0.25							Cycle Length, C = 100.0						
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		353	237	143	616					447	120		
Lane Group Capacity, c		710	624	479	914					654	606		
v/c Ratio, X		0.50	0.38	0.30	0.67					0.68	0.20		
Total Green Ratio, g/C		0.40	0.40	0.50	0.50					0.40	0.40		
Uniform Delay, d ₁		22.5	21.2	20.7	18.9					24.8	19.5		
Progression Factor, PF		1.000	1.000	0.926	0.333					1.000	1.000		
Delay Calibration, k		0.11	0.11	0.11	0.25					0.25	0.11		
Incremental Delay, d ₂		0.6	0.4	0.4	2.0					2.9	0.2		
Initial Queue Delay, d ₃		0.0	0.0	0.0	0.0					0.0	0.0		
Control Delay		23.0	21.6	19.6	8.3					27.7	19.7		
Lane Group LOS		C	C	B	A					C	B		
Approach Delay	22.5			10.4						26.0			
Approach LOS	C			B						C			
Intersection Delay	18.7			X _C = 0.68			Intersection LOS			B			

HCS+™ DETAILED REPORT

General Information				Site Information			
Analyst	JAG			Intersection	Rt 94/Rt 17 NB ramps		
Agency or Co.	TMA			Area Type	All other areas		
Date Performed	2/20/09			Jurisdiction	Village of Chester		
Time Period	PM Peak Hour			Analysis Year	Build Conndtion		
				Project ID			

Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _i	1	1			1	1	0	1	1			
Lane Group	L	T			T	R		LT	R			
Volume, V (vph)	220	406			353	367	343	1	227			
% Heavy Vehicles, %HV	4	4			2	2	5	5	5			
Peak-Hour Factor, PHF	0.88	0.88			0.96	0.96	0.83	0.83	0.83			
Pretimed (P) or Actuated (A)	A	A			A	A	A	A	A			
Start-up Lost Time, l _i	2.0	2.0			2.0	2.0		2.0	2.0			
Extension of Effective Green, e	2.0	2.0			2.0	2.0		2.0	2.0			
Arrival Type, AT	5	5			3	3		3	3			
Unit Extension, UE	3.0	3.0			3.0	3.0		3.0	3.0			
Filtering/Metering, I	1.000	1.000			1.000	1.000		1.000	1.000			
Initial Unmet Demand, Q _b	0.0	0.0			0.0	0.0		0.0	0.0			
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0	0	0	0	
Lane Width	12.0	12.0			11.0	13.0		12.0	13.0			
Parking / Grade / Parking	N	0	N	N	6	N	N	5	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0			0	0		0	0			
Min. Time for Pedestrians, G _p	3.2			3.2			3.2			3.2		
Phasing	EW Perm	EB Only	03	04	NB Only	06	07	08				
Timing	G = 50.0	G = 5.0	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 100.0					

Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	250	461			368	382		414	273			
Lane Group Capacity, c	592	1096			874	794		504	465			
v/c Ratio, X	0.42	0.42			0.42	0.48		0.82	0.59			
Total Green Ratio, g/C	0.60	0.60			0.50	0.50		0.30	0.30			
Uniform Delay, d ₁	15.3	10.7			15.8	16.5		32.5	29.7			
Progression Factor, PF	0.935	0.125			1.000	1.000		1.000	1.000			
Delay Calibration, k	0.11	0.11			0.11	0.11		0.36	0.18			
Incremental Delay, d ₂	0.5	0.3			0.3	0.5		10.5	1.9			
Initial Queue Delay, d ₃	0.0	0.0			0.0	0.0		0.0	0.0			
Control Delay	14.8	1.6			16.2	16.9		43.0	31.7			
Lane Group LOS	B	A			B	B		D	C			
Approach Delay	6.2			16.5			38.5					
Approach LOS	A			B			D					
Intersection Delay	20.2			X _C = 0.61			Intersection LOS			C		

HCS+™ DETAILED REPORT

General Information	Site Information
Analyst <i>JAG</i>	Intersection <i>NYS Rt. 94/ NYS Rt. 17M</i>
Agency or Co. <i>TMA</i>	Area Type <i>All other areas</i>
Date Performed <i>2/20/09</i>	Jurisdiction <i>Village of Chester</i>
Time Period <i>PM Peak Hour</i>	Analysis Year <i>Build Condition</i>
	Project ID

Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _l	1	1	1	1	1	0	2	1	0	1	2	0
Lane Group	L	T	R	L	TR		L	TR		L	TR	
Volume, V (vph)	272	225	189	50	207	102	312	397	13	99	440	317
% Heavy Vehicles, %HV	3	3	3	5	5	5	6	6	6	3	3	3
Peak-Hour Factor, PHF	0.95	0.95	0.95	0.92	0.92	0.92	0.94	0.94	0.94	0.91	0.91	0.91
Pretimed (P) or Actuated (A)	A	A	A	A	A	A	A	A	A	A	A	A
Start-up Lost Time, l ₁	2.0	2.0	2.0	2.0	2.0		2.0	2.0		2.0	2.0	
Extension of Effective Green, e	2.0	2.0	2.0	2.0	2.0		2.0	2.0		2.0	2.0	
Arrival Type, AT	3	3	3	3	3		3	3		3	3	
Unit Extension, UE	3.0	3.0	3.0	3.0	3.0		3.0	3.0		3.0	3.0	
Filtering/Metering, I	1.000	1.000	1.000	1.000	1.000		1.000	1.000		1.000	1.000	
Initial Unmet Demand, Q _b	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	105
Lane Width	12.0	12.0	12.0	12.0	12.0		10.0	10.0		10.0	12.0	
Parking / Grade / Parking	N	-5	N	N	-3	N	N	-5	N	N	5	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0	0	0	0		0	0		0	0	
Min. Time for Pedestrians, G _p	3.2			3.2			3.2			3.2		
Phasing	EW Perm	EB Only	03	04	Thru & RT	NB Only	SB Only	08				
Timing	G = 27.0	G = 12.0	G =	G =	G = 16.0	G = 14.0	G = 11.0	G =				
	Y = 4	Y = 4	Y =	Y =	Y = 4	Y = 4	Y = 4	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 100.0					

Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	286	237	199	54	336		332	436		109	717	
Lane Group Capacity, c	437	813	691	251	471		443	580		175	880	
v/c Ratio, X	0.65	0.29	0.29	0.22	0.71		0.75	0.75		0.62	0.81	
Total Green Ratio, g/C	0.43	0.43	0.43	0.27	0.27		0.14	0.34		0.11	0.27	
Uniform Delay, d ₁	32.5	18.6	18.5	28.3	33.0		41.3	29.3		42.5	34.2	
Progression Factor, PF	1.000	1.000	1.000	1.000	1.000		1.000	1.000		1.000	1.000	
Delay Calibration, k	0.23	0.11	0.11	0.11	0.28		0.30	0.31		0.21	0.36	
Incremental Delay, d ₂	3.5	0.2	0.2	0.4	5.1		7.0	5.5		6.7	6.0	
Initial Queue Delay, d ₃	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Control Delay	36.0	18.8	18.8	28.7	38.1		48.3	34.7		49.2	40.1	
Lane Group LOS	D	B	B	C	D		D	C		D	D	
Approach Delay	25.6			36.8			40.6			41.3		
Approach LOS	C			D			D			D		
Intersection Delay	36.3			X _C = 0.76			Intersection LOS			D		

TWO-WAY STOP CONTROL SUMMARY								
General Information				Site Information				
Analyst	JAG			Intersection	Hambletonian and High			
Agency/Co.	TMA			Jurisdiction	Chester			
Date Performed	2/17/2009			Analysis Year	Build Condition			
Analysis Time Period	PM Peak Hour							
Project Description								
East/West Street: Hambletonian Avenue				North/South Street: High Street				
Intersection Orientation: North-South				Study Period (hrs): 0.25				
Vehicle Volumes and Adjustments								
Major Street	Northbound			Southbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	16	307			210	16		
Peak-Hour Factor, PHF	0.93	0.93	1.00	1.00	0.90	0.90		
Hourly Flow Rate, HFR (veh/h)	17	330	0	0	233	17		
Percent Heavy Vehicles	1	--	--	0	--	--		
Median Type	Undivided							
RT Channelized			0			0		
Lanes	0	1	0	0	1	0		
Configuration	LT						TR	
Upstream Signal		0			0			
Minor Street	Eastbound			Westbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)	38		16					
Peak-Hour Factor, PHF	0.64	1.00	0.64	1.00	1.00	1.00		
Hourly Flow Rate, HFR (veh/h)	59	0	25	0	0	0		
Percent Heavy Vehicles	4	0	4	0	0	0		
Percent Grade (%)	0			0				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	0		
Configuration		LR						
Delay, Queue Length, and Level of Service								
Approach	Northbound	Southbound	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	LT						LR	
v (veh/h)	17						84	
C (m) (veh/h)	1321						517	
v/c	0.01						0.16	
95% queue length	0.04						0.58	
Control Delay (s/veh)	7.8						13.3	
LOS	A						B	
Approach Delay (s/veh)	--	--					13.3	
Approach LOS	--	--					B	

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	JAG			Intersection	Ward and Rt 17M		
Agency/Co.	TMA			Jurisdiction	Goshen		
Date Performed	2/17/2009			Analysis Year	Build Condition		
Analysis Time Period	PM Peak Hour						
Project Description							
East/West Street: Ward Road				North/South Street: NYS Route 17M			
Intersection Orientation: North-South				Study Period (hrs): 0.25			
Vehicle Volumes and Adjustments							
Major Street		Northbound			Southbound		
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		454	15	57	333		
Peak-Hour Factor, PHF	1.00	0.93	0.93	0.84	0.84	1.00	
Hourly Flow Rate, HFR (veh/h)	0	488	16	67	396	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Undivided						
RT Channelized			0			0	
Lanes	0	1	0	0	1	0	
Configuration			TR	LT			
Upstream Signal		0			0		
Minor Street		Eastbound			Westbound		
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				5		50	
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.81	1.00	0.81	
Hourly Flow Rate, HFR (veh/h)	0	0	0	6	0	61	
Percent Heavy Vehicles	0	0	0	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0			0	
Lanes	0	0	0	0	0	0	
Configuration					LR		
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration		LT		LR			
v (veh/h)		67		67			
C (m) (veh/h)		1071		516			
v/c		0.06		0.13			
95% queue length		0.20		0.44			
Control Delay (s/veh)		8.6		13.0			
LOS		A		B			
Approach Delay (s/veh)	--	--	13.0				
Approach LOS	--	--	B				

TWO-WAY STOP CONTROL SUMMARY								
General Information				Site Information				
Analyst	JAG			Intersection	Rt 17M and Main			
Agency/Co.	TMA			Jurisdiction	Chester			
Date Performed	2/17/2009			Analysis Year	Build Condition			
Analysis Time Period	PM Peak Hour							
Project Description								
East/West Street: NYS Route 17M				North/South Street: Main Street				
Intersection Orientation: East-West				Study Period (hrs): 0.25				
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	20	615			630	134		
Peak-Hour Factor, PHF	0.84	0.84	1.00	1.00	0.95	0.95		
Hourly Flow Rate, HFR (veh/h)	23	732	0	0	663	141		
Percent Heavy Vehicles	3	--	--	0	--	--		
Median Type	Two Way Left Turn Lane							
RT Channelized			0			0		
Lanes	1	1	0	0	1	0		
Configuration	L	T				TR		
Upstream Signal		0			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)				115		16		
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.60	1.00	0.60		
Hourly Flow Rate, HFR (veh/h)	0	0	0	191	0	26		
Percent Heavy Vehicles	0	0	0	3	0	3		
Percent Grade (%)	0			0				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	0		
Configuration					LR			
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	L						LR	
v (veh/h)	23						217	
C (m) (veh/h)	816						337	
v/c	0.03						0.64	
95% queue length	0.09						4.23	
Control Delay (s/veh)	9.5						33.1	
LOS	A						D	
Approach Delay (s/veh)	--	--					33.1	
Approach LOS	--	--					D	

HCS+™ DETAILED REPORT

General Information		Site Information	
Analyst	JAG	Intersection	Arcadia and Rt 17M
Agency or Co.	TMA	Area Type	All other areas
Date Performed	2/17/2009	Jurisdiction	Goshen
Time Period	PM Peak Hour	Analysis Year	Build Condition
		Project ID	

Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _l	0		0				0	1			1	0
Lane Group		LR						LT			TR	
Volume, V (vph)	51		77				77	425			342	61
% Heavy Vehicles, %HV	0		0				1	1			0	0
Peak-Hour Factor, PHF	0.91		0.91				0.88	0.88			0.79	0.79
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, l ₁		2.0						2.0			2.0	
Extension of Effective Green, e		2.0						2.0			2.0	
Arrival Type, AT		3						3			3	
Unit Extension, UE		3.0						3.0			3.0	
Filtering/Metering, I		1.000						1.000			1.000	
Initial Unmet Demand, Q _b		0.0						0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0
Lane Width		16.0						12.0			12.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b		0						0			0	
Min. Time for Pedestrians, G _p		3.2			3.2			3.2			3.2	
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 20.0	G =	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					

Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		141						571			510	
Lane Group Capacity, c		647						755			931	
v/c Ratio, X		0.22						0.76			0.55	
Total Green Ratio, g/C		0.33						0.50			0.50	
Uniform Delay, d ₁		14.4						12.1			10.3	
Progression Factor, PF		1.000						1.000			1.000	
Delay Calibration, k		0.50						0.50			0.50	
Incremental Delay, d ₂		0.8						7.0			2.3	
Initial Queue Delay, d ₃		0.0						0.0			0.0	
Control Delay		15.2						19.0			12.6	
Lane Group LOS		B						B			B	
Approach Delay		15.2						19.0			12.6	
Approach LOS		B						B			B	
Intersection Delay		15.9			X _C = 0.54			Intersection LOS			B	

HCS+™ DETAILED REPORT

General Information	Site Information
Analyst <i>JAG</i>	Intersection <i>West Ave./ Route 17M</i>
Agency or Co. <i>TMA</i>	Area Type <i>All other areas</i>
Date Performed <i>2/9/2009</i>	Jurisdiction <i>Village of Chester</i>
Time Period <i>PM Peak Hour</i>	Analysis Year <i>Build Condition</i>
	Project ID

Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N _i	0	1	1	1	1	0	1	1	1	1	1	0	
Lane Group		<i>LT</i>	<i>R</i>	<i>L</i>	<i>TR</i>		<i>L</i>	<i>T</i>	<i>R</i>	<i>L</i>	<i>TR</i>		
Volume, V (vph)	129	76	132	247	108	50	119	351	249	87	289	67	
% Heavy Vehicles, %HV	3	3	3	1	1	1	0	0	0	0	0	0	
Peak-Hour Factor, PHF	0.81	0.81	0.81	0.88	0.88	0.88	0.94	0.94	0.94	0.98	0.98	0.98	
Pretimed (P) or Actuated (A)	A	A	A	A	A	A	A	A	A	A	A	A	
Start-up Lost Time, l _i		2.0	2.0	2.0	2.0		2.0	2.0	2.0	2.0	2.0		
Extension of Effective Green, e		2.0	2.0	2.0	2.0		2.0	2.0	2.0	2.0	2.0		
Arrival Type, AT		3	3	3	3		3	3	3	3	3		
Unit Extension, UE		3.0	3.0	3.0	3.0		3.0	3.0	3.0	3.0	3.0		
Filtering/Metering, I		1.000	1.000	1.000	1.000		1.000	1.000	1.000	1.000	1.000		
Initial Unmet Demand, Q _b		0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0	
Lane Width		11.0	12.0	14.0	12.0		11.0	11.0	15.0	13.0	13.0		
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N	
Parking Maneuvers, N _m													
Buses Stopping, N _b		0	0	0	0		0	0	0	0	0		
Min. Time for Pedestrians, G _p		12.4			16.2			15.4			14.9		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08					
Timing	G = 16.0	G =	G =	G =	G = 29.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 55.0						

Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		253	163	281	180		127	373	265	89	363		
Lane Group Capacity, c		366	456	321	521		499	969	937	524	1006		
v/c Ratio, X		0.69	0.36	0.88	0.35		0.25	0.38	0.28	0.17	0.36		
Total Green Ratio, g/C		0.29	0.29	0.29	0.29		0.53	0.53	0.53	0.53	0.53		
Uniform Delay, d ₁		17.3	15.4	18.6	15.4		7.1	7.7	7.2	6.7	7.6		
Progression Factor, PF		1.000	1.000	1.000	1.000		1.000	1.000	1.000	1.000	1.000		
Delay Calibration, k		0.26	0.11	0.40	0.11		0.11	0.11	0.11	0.11	0.11		
Incremental Delay, d ₂		5.5	0.5	22.6	0.4		0.3	0.3	0.2	0.2	0.2		
Initial Queue Delay, d ₃		0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0		
Control Delay		22.8	15.9	41.2	15.8		7.4	8.0	7.4	6.9	7.8		
Lane Group LOS		C	B	D	B		A	A	A	A	A		
Approach Delay		20.1			31.3			7.7			7.6		
Approach LOS		C			C			A			A		
Intersection Delay		15.3			X _C = 0.56			Intersection LOS			B		

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	JAG			Intersection	Site and Rte 17M		
Agency/Co.	TMA			Jurisdiction	Chester		
Date Performed	2/25/2009			Analysis Year	Build Condition		
Analysis Time Period							
Project Description							
East/West Street: Site Access				North/South Street: NYS Route 17M			
Intersection Orientation: North-South				Study Period (hrs): 0.25			
Vehicle Volumes and Adjustments							
Major Street		Northbound			Southbound		
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		488	125	30	330		
Peak-Hour Factor, PHF	1.00	0.94	0.94	0.86	0.86	1.00	
Hourly Flow Rate, HFR (veh/h)	0	519	132	34	383	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Undivided						
RT Channelized			0			0	
Lanes	0	1	0	0	1	0	
Configuration			TR	LT			
Upstream Signal		0			0		
Minor Street		Eastbound			Westbound		
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				72		18	
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.90	1.00	0.90	
Hourly Flow Rate, HFR (veh/h)	0	0	0	80	0	20	
Percent Heavy Vehicles	0	0	0	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0			0	
Lanes	0	0	0	0	0	0	
Configuration					LR		
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration		LT		LR			
v (veh/h)		34		100			
C (m) (veh/h)		945		279			
v/c		0.04		0.36			
95% queue length		0.11		1.57			
Control Delay (s/veh)		9.0		24.9			
LOS		A		C			
Approach Delay (s/veh)	--	--	24.9				
Approach LOS	--	--	C				

HCS+™ DETAILED REPORT

General Information	Site Information
Analyst <i>JAG</i>	Intersection <i>Route 94/Rt 17 SB ramp</i>
Agency or Co. <i>TMA</i>	Area Type <i>All other areas</i>
Date Performed <i>2/20/09</i>	Jurisdiction <i>Village of Chester</i>
Time Period <i>Saturday Peak Hour</i>	Analysis Year <i>Build Condition</i>
	Project ID

Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N _l		1	1	1	1					1	1	0	
Lane Group		T	R	L	T					L	TR		
Volume, V (vph)		235	157	168	345					303	1	106	
% Heavy Vehicles, %HV		2	2	1	1					2	2	2	
Peak-Hour Factor, PHF		0.86	0.86	0.91	0.91					0.89	0.89	0.89	
Pretimed (P) or Actuated (A)		A	A	A	A					A	A	A	
Start-up Lost Time, l ₁		2.0	2.0	2.0	2.0					2.0	2.0		
Extension of Effective Green, e		2.0	2.0	2.0	2.0					2.0	2.0		
Arrival Type, AT		3	3	5	5					3	3		
Unit Extension, UE		3.0	3.0	3.0	3.0					3.0	3.0		
Filtering/Metering, I		1.000	1.000	1.000	1.000					1.000	1.000		
Initial Unmet Demand, Q _b		0.0	0.0	0.0	0.0					0.0	0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0	
Lane Width		12.0	13.0	12.0	12.0					11.0	12.0		
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	5	N	
Parking Maneuvers, N _m													
Buses Stopping, N _b		0	0	0	0					0	0		
Min. Time for Pedestrians, G _p		3.2			3.2			3.2			3.2		
Phasing	EW Perm	WB Only	03		04		SB Only	06		07		08	
Timing	G = 40.0	G = 5.0	G =		G =		G = 40.0	G =		G =		G =	
	Y = 5	Y = 5	Y =		Y =		Y = 5	Y =		Y =		Y =	
Duration of Analysis, T = 0.25							Cycle Length, C = 100.0						

Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		273	183	185	379					340	120		
Lane Group Capacity, c		745	654	560	941					667	618		
v/c Ratio, X		0.37	0.28	0.33	0.40					0.51	0.19		
Total Green Ratio, g/C		0.40	0.40	0.50	0.50					0.40	0.40		
Uniform Delay, d ₁		21.1	20.3	18.6	15.7					22.6	19.5		
Progression Factor, PF		1.000	1.000	0.926	0.333					1.000	1.000		
Delay Calibration, k		0.11	0.11	0.11	0.11					0.12	0.11		
Incremental Delay, d ₂		0.3	0.2	0.3	0.3					0.7	0.2		
Initial Queue Delay, d ₃		0.0	0.0	0.0	0.0					0.0	0.0		
Control Delay		21.4	20.5	17.6	5.5					23.3	19.7		
Lane Group LOS		C	C	B	A					C	B		
Approach Delay		21.0			9.5						22.3		
Approach LOS		C			A						C		
Intersection Delay		17.0			X _C = 0.45			Intersection LOS			B		

HCS+™ DETAILED REPORT

General Information	Site Information
Analyst <i>JAG</i>	Intersection <i>Rt 94/Rt 17 NB ramps</i>
Agency or Co. <i>TMA</i>	Area Type <i>All other areas</i>
Date Performed <i>2/20/09</i>	Jurisdiction <i>Village of Chester</i>
Time Period <i>Saturday Peak Hour</i>	Analysis Year <i>Build Condition</i>
	Project ID

Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _l	1	1			1	1	0	1	1			
Lane Group	L	T			T	R		LT	R			
Volume, V (vph)	84	449			338	363	182	0	182			
% Heavy Vehicles, %HV	1	1			1	1	2	2	2			
Peak-Hour Factor, PHF	0.97	0.97			0.85	0.85	0.94	0.94	0.94			
Pretimed (P) or Actuated (A)	A	A			A	A	A	A	A			
Start-up Lost Time, l ₁	2.0	2.0			2.0	2.0		2.0	2.0			
Extension of Effective Green, e	2.0	2.0			2.0	2.0		2.0	2.0			
Arrival Type, AT	5	5			3	3		3	3			
Unit Extension, UE	3.0	3.0			3.0	3.0		3.0	3.0			
Filtering/Metering, I	1.000	1.000			1.000	1.000		1.000	1.000			
Initial Unmet Demand, Q _b	0.0	0.0			0.0	0.0		0.0	0.0			
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0	0	0	0	
Lane Width	12.0	12.0			11.0	13.0		12.0	13.0			
Parking / Grade / Parking	N	0	N	N	6	N	N	5	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0			0	0		0	0			
Min. Time for Pedestrians, G _p	3.2			3.2			3.2			3.2		
Phasing	EW Perm	EB Only	03	04	NB Only	06	07	08				
Timing	G = 50.0	G = 5.0	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 100.0					

Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	87	463			398	427		194	194			
Lane Group Capacity, c	584	1129			882	802		519	479			
v/c Ratio, X	0.15	0.41			0.45	0.53		0.37	0.41			
Total Green Ratio, g/C	0.60	0.60			0.50	0.50		0.30	0.30			
Uniform Delay, d ₁	13.3	10.6			16.1	17.0		27.6	27.9			
Progression Factor, PF	0.935	0.125			1.000	1.000		1.000	1.000			
Delay Calibration, k	0.11	0.11			0.11	0.14		0.11	0.11			
Incremental Delay, d ₂	0.1	0.2			0.4	0.7		0.5	0.6			
Initial Queue Delay, d ₃	0.0	0.0			0.0	0.0		0.0	0.0			
Control Delay	12.5	1.6			16.5	17.7		28.0	28.5			
Lane Group LOS	B	A			B	B		C	C			
Approach Delay	3.3			17.1			28.2					
Approach LOS	A			B			C					
Intersection Delay	15.3			X _C = 0.43			Intersection LOS			B		

HCS+™ DETAILED REPORT

General Information	Site Information
Analyst <i>JAG</i>	Intersection <i>NYS Rt. 94/ NYS Rt. 17M</i>
Agency or Co. <i>TMA</i>	Area Type <i>All other areas</i>
Date Performed <i>2/20/09</i>	Jurisdiction <i>Village of Chester</i>
Time Period <i>Saturday Peak Hour</i>	Analysis Year <i>Build Condition</i>
	Project ID

Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _l	1	1	1	1	1	0	2	1	0	1	2	0
Lane Group	L	T	R	L	TR		L	TR		L	TR	
Volume, V (vph)	309	182	291	65	193	130	335	370	28	135	299	240
% Heavy Vehicles, %HV	3	3	3	3	3	3	1	1	1	2	2	2
Peak-Hour Factor, PHF	0.87	0.87	0.87	0.86	0.86	0.86	0.94	0.94	0.94	0.94	0.94	0.94
Pretimed (P) or Actuated (A)	A	A	A	A	A	A	A	A	A	A	A	A
Start-up Lost Time, l ₁	2.0	2.0	2.0	2.0	2.0		2.0	2.0		2.0	2.0	
Extension of Effective Green, e	2.0	2.0	2.0	2.0	2.0		2.0	2.0		2.0	2.0	
Arrival Type, AT	3	3	3	3	3		3	3		3	3	
Unit Extension, UE	3.0	3.0	3.0	3.0	3.0		3.0	3.0		3.0	3.0	
Filtering/Metering, I	1.000	1.000	1.000	1.000	1.000		1.000	1.000		1.000	1.000	
Initial Unmet Demand, Q _b	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	80
Lane Width	12.0	12.0	12.0	12.0	12.0		10.0	10.0		10.0	12.0	
Parking / Grade / Parking	N	-5	N	N	-3	N	N	-5	N	N	5	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0	0	0	0		0	0		0	0	
Min. Time for Pedestrians, G _p	3.2			3.2			3.2			3.2		
Phasing	EW Perm	EB Only	03	04	NS Perm	NB Only	SB Only	08				
Timing	G = 25.0	G = 19.0	G =	G =	G = 12.0	G = 14.0	G = 10.0	G =				
	Y = 4	Y = 4	Y =	Y =	Y = 4	Y = 4	Y = 4	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 100.0					

Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	355	209	334	76	375		356	424		144	488	
Lane Group Capacity, c	507	908	771	260	440		465	534		233	721	
v/c Ratio, X	0.70	0.23	0.43	0.29	0.85		0.77	0.79		0.62	0.68	
Total Green Ratio, g/C	0.48	0.48	0.48	0.25	0.25		0.14	0.30		0.22	0.22	
Uniform Delay, d ₁	31.6	15.2	17.1	30.3	35.7		41.4	32.2		40.5	35.7	
Progression Factor, PF	1.000	1.000	1.000	1.000	1.000		1.000	1.000		1.000	1.000	
Delay Calibration, k	0.27	0.11	0.11	0.11	0.38		0.32	0.34		0.20	0.25	
Incremental Delay, d ₂	4.3	0.1	0.4	0.6	14.8		7.5	8.1		4.9	2.5	
Initial Queue Delay, d ₃	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Control Delay	35.8	15.3	17.5	31.0	50.6		48.9	40.3		45.4	38.3	
Lane Group LOS	D	B	B	C	D		D	D		D	D	
Approach Delay	24.2			47.3			44.2			39.9		
Approach LOS	C			D			D			D		
Intersection Delay	37.2			X _C = 0.74			Intersection LOS			D		

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	JAG			Intersection	Hambletonian and High		
Agency/Co.	TMA			Jurisdiction	Chester		
Date Performed	2/17/2009			Analysis Year	Build Condition		
Analysis Time Period	Saturday Peak Hour						
Project Description							
East/West Street: Hambletonian Avenue				North/South Street: High Street			
Intersection Orientation: North-South				Study Period (hrs): 0.25			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)	19	283			307	30	
Peak-Hour Factor, PHF	0.91	0.91	1.00	1.00	0.88	0.88	
Hourly Flow Rate, HFR (veh/h)	20	310	0	0	348	34	
Percent Heavy Vehicles	1	--	--	0	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	1	0	
Configuration	LT						TR
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)	42		24				
Peak-Hour Factor, PHF	0.70	1.00	0.70	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	60	0	34	0	0	0	
Percent Heavy Vehicles	4	0	4	0	0	0	
Percent Grade (%)	0			0			
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0			0	
Lanes	0	0	0	0	0	0	
Configuration		LR					
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration	LT						LR
v (veh/h)	20						94
C (m) (veh/h)	1182						458
v/c	0.02						0.21
95% queue length	0.05						0.76
Control Delay (s/veh)	8.1						14.9
LOS	A						B
Approach Delay (s/veh)	--	--					14.9
Approach LOS	--	--					B

TWO-WAY STOP CONTROL SUMMARY								
General Information				Site Information				
Analyst	JAG			Intersection	Ward and Rt 17M			
Agency/Co.	TMA			Jurisdiction	Goshen			
Date Performed	2/17/2009			Analysis Year	Build Condition			
Analysis Time Period	Saturday Peak Hour							
Project Description								
East/West Street: Ward Road				North/South Street: NYS Route 17M				
Intersection Orientation: North-South				Study Period (hrs): 0.25				
Vehicle Volumes and Adjustments								
Major Street	Northbound			Southbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)		374	7	30	296			
Peak-Hour Factor, PHF	1.00	0.91	0.91	0.86	0.86	1.00		
Hourly Flow Rate, HFR (veh/h)	0	410	7	34	344	0		
Percent Heavy Vehicles	0	--	--	0	--	--		
Median Type	Undivided							
RT Channelized			0			0		
Lanes	0	1	0	0	1	0		
Configuration			TR	LT				
Upstream Signal		0			0			
Minor Street	Eastbound			Westbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)				2		14		
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.63	1.00	0.63		
Hourly Flow Rate, HFR (veh/h)	0	0	0	3	0	22		
Percent Heavy Vehicles	0	0	0	0	0	0		
Percent Grade (%)	0			0				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	0		
Configuration					LR			
Delay, Queue Length, and Level of Service								
Approach	Northbound	Southbound	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration		LT		LR				
v (veh/h)		34		25				
C (m) (veh/h)		1153		579				
v/c		0.03		0.04				
95% queue length		0.09		0.14				
Control Delay (s/veh)		8.2		11.5				
LOS		A		B				
Approach Delay (s/veh)	--	--	11.5					
Approach LOS	--	--	B					

TWO-WAY STOP CONTROL SUMMARY								
General Information				Site Information				
Analyst	JAG			Intersection	Rt 17M and Main			
Agency/Co.	TMA			Jurisdiction	Chester			
Date Performed	2/17/2009			Analysis Year	Build Condition			
Analysis Time Period	Saturday Peak Hour							
Project Description								
East/West Street: NYS Route 17M				North/South Street: Main Street				
Intersection Orientation: East-West				Study Period (hrs): 0.25				
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	17	450			613	99		
Peak-Hour Factor, PHF	0.95	0.95	1.00	1.00	0.98	0.98		
Hourly Flow Rate, HFR (veh/h)	17	473	0	0	625	101		
Percent Heavy Vehicles	3	--	--	0	--	--		
Median Type	Two Way Left Turn Lane							
RT Channelized			0			0		
Lanes	1	1	0	0	1	0		
Configuration	L	T				TR		
Upstream Signal		0			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)				65		36		
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.98	1.00	0.98		
Hourly Flow Rate, HFR (veh/h)	0	0	0	66	0	36		
Percent Heavy Vehicles	0	0	0	2	0	2		
Percent Grade (%)	0			0				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	0		
Configuration					LR			
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	L						LR	
v (veh/h)	17						102	
C (m) (veh/h)	872						423	
v/c	0.02						0.24	
95% queue length	0.06						0.93	
Control Delay (s/veh)	9.2						16.2	
LOS	A						C	
Approach Delay (s/veh)	--	--					16.2	
Approach LOS	--	--					C	

HCS+™ DETAILED REPORT

General Information	Site Information
Analyst JAG	Intersection Arcadia and Rt 17M
Agency or Co. TMA	Area Type All other areas
Date Performed 2/17/2009	Jurisdiction Goshen
Time Period Saturday Peak Hour	Analysis Year Build Condition
	Project ID

Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _l	0		0				0	1			1	0
Lane Group		LR						LT			TR	
Volume, V (vph)	45		50				57	302			241	54
% Heavy Vehicles, %HV	0		0				0	0			0	0
Peak-Hour Factor, PHF	0.94		0.94				0.96	0.96			0.80	0.80
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, l ₁		2.0						2.0			2.0	
Extension of Effective Green, e		2.0						2.0			2.0	
Arrival Type, AT		3						3			3	
Unit Extension, UE		3.0						3.0			3.0	
Filtering/Metering, I		1.000						1.000			1.000	
Initial Unmet Demand, Q _b		0.0						0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0
Lane Width		16.0						12.0			12.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b		0						0			0	
Min. Time for Pedestrians, G _p		3.2			3.2			3.2			3.2	
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 20.0	G =	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					

Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		101						374			368	
Lane Group Capacity, c		651						853			927	
v/c Ratio, X		0.16						0.44			0.40	
Total Green Ratio, g/C		0.33						0.50			0.50	
Uniform Delay, d ₁		14.1						9.6			9.4	
Progression Factor, PF		1.000						1.000			1.000	
Delay Calibration, k		0.50						0.50			0.50	
Incremental Delay, d ₂		0.5						1.6			1.3	
Initial Queue Delay, d ₃		0.0						0.0			0.0	
Control Delay		14.6						11.2			10.6	
Lane Group LOS		B						B			B	
Approach Delay		14.6						11.2			10.6	
Approach LOS		B						B			B	
Intersection Delay		11.4			X _C = 0.33			Intersection LOS			B	

HCS+™ DETAILED REPORT

General Information				Site Information			
Analyst	JAG			Intersection	West Ave./ Route 17M		
Agency or Co.	TMA			Area Type	All other areas		
Date Performed	2/9/2009			Jurisdiction	Village of Chester		
Time Period	Saturday Peak Hour			Analysis Year	Build Condition		
				Project ID			

Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N _i	0	1	1	1	1	0	1	1	1	1	1	0	
Lane Group	LT	R	L	TR			L	T	R	L	TR		
Volume, V (vph)	77	126	103	273	119	36	81	333	380	106	236	81	
% Heavy Vehicles, %HV	3	3	3	1	1	1	2	2	2	2	2	2	
Peak-Hour Factor, PHF	0.81	0.81	0.81	0.95	0.95	0.95	0.95	0.95	0.95	0.92	0.92	0.92	
Pretimed (P) or Actuated (A)	A	A	A	A	A	A	A	A	A	A	A	A	
Start-up Lost Time, I ₁		2.0	2.0	2.0	2.0		2.0	2.0	2.0	2.0	2.0		
Extension of Effective Green, e		2.0	2.0	2.0	2.0		2.0	2.0	2.0	2.0	2.0		
Arrival Type, AT		3	3	3	3		3	3	3	3	3		
Unit Extension, UE		3.0	3.0	3.0	3.0		3.0	3.0	3.0	3.0	3.0		
Filtering/Metering, I		1.000	1.000	1.000	1.000		1.000	1.000	1.000	1.000	1.000		
Initial Unmet Demand, Q _b		0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0	
Lane Width		11.0	12.0	14.0	12.0		11.0	11.0	15.0	13.0	13.0		
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N	
Parking Maneuvers, N _m													
Buses Stopping, N _b		0	0	0	0		0	0	0	0	0		
Min. Time for Pedestrians, G _p		12.4			16.2			15.4			14.9		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08					
Timing	G = 16.0	G =	G =	G =	G = 29.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 55.0						

Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		251	127	287	163		85	351	400	115	345		
Lane Group Capacity, c		419	456	323	528		505	950	919	534	976		
v/c Ratio, X		0.60	0.28	0.89	0.31		0.17	0.37	0.44	0.22	0.35		
Total Green Ratio, g/C		0.29	0.29	0.29	0.29		0.53	0.53	0.53	0.53	0.53		
Uniform Delay, d ₁		16.7	15.0	18.6	15.2		6.7	7.6	8.0	6.9	7.6		
Progression Factor, PF		1.000	1.000	1.000	1.000		1.000	1.000	1.000	1.000	1.000		
Delay Calibration, k		0.19	0.11	0.41	0.11		0.11	0.11	0.11	0.11	0.11		
Incremental Delay, d ₂		2.4	0.3	24.6	0.3		0.2	0.2	0.3	0.2	0.2		
Initial Queue Delay, d ₃		0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0		
Control Delay		19.1	15.4	43.3	15.5		6.9	7.9	8.3	7.1	7.8		
Lane Group LOS		B	B	D	B		A	A	A	A	A		
Approach Delay		17.9			33.2			8.0			7.6		
Approach LOS		B			C			A			A		
Intersection Delay		15.0			X _C = 0.60			Intersection LOS			B		

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	JAG			Intersection	Site and Rte 17M		
Agency/Co.	TMA			Jurisdiction	Chester		
Date Performed	2/25/2009			Analysis Year	Build Condition		
Analysis Time Period	Saturday Peak Hour						
Project Description							
East/West Street: Site Access				North/South Street: NYS Route 17M			
Intersection Orientation: North-South				Study Period (hrs): 0.25			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		405	99	18	305		
Peak-Hour Factor, PHF	1.00	0.92	0.92	0.92	0.92	1.00	
Hourly Flow Rate, HFR (veh/h)	0	440	107	19	331	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	1	0	
Configuration			TR	LT			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				83		19	
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.90	1.00	0.90	
Hourly Flow Rate, HFR (veh/h)	0	0	0	92	0	21	
Percent Heavy Vehicles	0	0	0	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0			0	
Lanes	0	0	0	0	0	0	
Configuration					LR		
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration		LT		LR			
v (veh/h)		19		113			
C (m) (veh/h)		1033		351			
v/c		0.02		0.32			
95% queue length		0.06		1.36			
Control Delay (s/veh)		8.6		20.1			
LOS		A		C			
Approach Delay (s/veh)	--	--	20.1				
Approach LOS	--	--	C				